CODES AND STANDARDS EDITION: FLORIDA BUILDING CODE (SIXTH EDITION) 2017.

- ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMMENTARY. AISC, "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- AISC, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
- AISC, "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". 6. SJI, "STANDARD SPECIFICATIONS FOR K SERIES, LH SERIES, AND DLH SERIES OPEN WEB STEEL JOISTS AND JOIST GIRDERS".
- SDI. "FLOOR DECK DESIGN MANUAL" AND "ROOF DECK DESIGN MANUAL".
- TMS, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", TMS 402/602-16. AISI, "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS",
- 10. AISI, "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", AISI SIOO-12.
- NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, 2015 EDITION ". AITC, "TIMBER CONSTRUCTION MANUAL, 6TH EDITION, 2012".

GENERAL:

1. THE CONTRACTOR SHALL VERIFY THE LOCATION OF UTILITIES IN THE AREA OF THE CONSTRUCTION. THE CONTRACTOR SHALL REPAIR OR REPLACE ANY UTILITIES DAMAGED DURING CONSTRUCTION TO THE

- SATISFACTION OF THE OWNER OF THE UTILITY. THE CONTRACTOR SHALL REPAIR OR REPLACE EXISTING BUILDINGS AND STRUCTURES ADJACENT TO THE CONSTRUCTION IF DAMAGED BY THE CONTRACTOR DURING CONSTRUCTION.
- SHOP DRAWINGS AS REQUIRED BY THE CONTRACT DOCUMENTS SHALL BE SUBMITTED BY THE CONTRACTOR PRIOR TO THE CONSTRUCTION OF ANY ELEMENTS INCLUDED IN THE SHOP DRAWINGS. THE ARCHITECT/ENGINEER WILL NOT CERTIFY CONSTRUCTION FOR ELEMENTS REQUIRING SHOP DRAWINGS UNLESS AND UNTIL THE SHOP DRAWINGS ARE REVIEWED AND APPROVED BY THE ARCHITECT/ENGINEER
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION AND NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR CONFLICTS. LOCATIONS AND SIZES OF OPENINGS, SLEEVES AND ANCHORAGE FOR ELECTRICAL AND MECHANICAL
- EQUIPMENT SHOWN ON THE STRUCTURAL PLANS ARE APPROXIMATE. THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHORAGE WITH THE TRADES FURNISHING ALL ELEVATIONS ARE BASED ON THE FINISHED FLOOR ELEVATION OF THE GROUND FLOOR TO BE 0'-0".
- THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF ALL DEPRESSIONS IN CONCRETE SLABS. THIS STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER IT'S BEEN FULLY COMPLETED. IT'S THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF WORKERS, THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE

ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, ETC. THAT MAY BE NECESSARY. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS. CONTRACTOR PROPOSED CHANGES AND SUBSTITUTIONS:

PROPOSED CHANGES OR SUBSTITUTIONS TO STRUCTURAL DETAILS OR PLANS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD (EOR) FOR REVIEW AND APPROVAL. SUBMITTALS SHALL CONTAIN FULL DOCUMENTATION OF CHANGES OR SUBSTITUTIONS WITH SUPPORTING, SEALED CALCULATIONS (WHERE APPLICABLE). THE REVIEW OF CHANGES AND SUBSTITUTIONS, RE-ANALYSIS AND/OR RE-DRAFTING TO INCORPORATE CHANGES OR SUBSTITUTIONS INTO CONTRACT DOCUMENTS ARE ADDITIONAL SERVICES FOR EOR. CONSTRUCTION COST REVISIONS ARE BETWEEN THE CONTRACTOR AND OWNER AND ARE NOT REVIEWED BY THE EOR.

- UNLEGS OTHERWISE NOTED, THE OWNER SHALL ENGAGE A CERTIFIED INDEPENDENT TESTING
 LABORATORY THAT IS ACCEPTABLE TO THE ARCHITECT/ENGINEER TO PERFORM TESTS AND INSPECTIONS AND SUBMIT REPORTS OF THE RESULTS AS REQUIRED IN THE PROJECT SPECIFICATIONS. THIS INCLUDES, BUT
- INSPECTION OF BEARING CAPACITY OF FOUNDATION SOILS
- B. INSPECTION OF COMPACTION OF FILLS INSPECTION OF REINFORCED CONCRETE CONSTRUCTION AS SHOWN IN THE "REINFORCED CONCRETE
- INSPECTIONS TABLE" ON THIS DRAWING D. INSPECTION OF STRUCTURAL MASONRY CONSTRUCTION AS SHOWN IN THE "STRUCTURAL MASONRY
- INSPECTIONS TABLE" ON HIS DRAWING.
- INSPECTION OF STRUCTURAL STEEL CONSTRUCTION AS SHOWN IN THE "MANDATORY STRUCTURAL STEEL INSPECTIONS TABLE" ON THIS DRAWING.
- THE TESTING AND INSPECTIONS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A QUALIFIED PROFESSIONAL ENGINEER CURRENTLY REGISTERED IN FLORIDA.
- ALL TECHNICIANS AND INSPECTORS SHALL BE CERTIFIED BY AGENCIES RECOGNIZED BY THE STATE OF FLORIDA AS QUALIFIED TO PERFORM THE TESTS AND INSPECTIONS WHICH THEY PERFORM.

FOUNDATIONS ARE DESIGNED FOR AN ASSUMED MAXIMUM NET SOIL BEARING PRESSURE OF 2000 PSF ALL SOIL BELOW THE BUILDING FOOTPRINT SHALL BE TREATED FOR TERMITE CONTROL PRIOR TO

ALL SLABS ON GRADE SHALL BE PLACED ON A 10 MIL VAPOR BARRIER AND TERMITE TREATED SOIL. INDOOR SLABS SHALL BE 4" THICK. PARKING GARAGE SLAB SHALL BE MIN. 6" THICK AND SLOPED TO

REINFORCED CONCRETE:

REQUIRED CONCRETE COMPRESSIVE STRENGTHS AT 28 DAYS, UNLESS OTHERWISE NOTED: SIDEWALKS, SLABS ON GROUND, & FOUNDATIONS f'c = 3*000* PSI

- COLUMNS, BEAMS, SHEAR WALLS & ELEVATED SLABS, TIE BEAMS f'c = 4000 PSI MINIMUM CONCRETE COVER CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH3" CONCRETE EXPOSED TO EARTH OR WEATHER: NO. 6 THROUGH NO. 18 BAR . . NO. 5 BAR, W31 OR D31 WIRE AND SMALLER CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:
- SLABS, WALLS & JOISTS: NO. 14 \$ NO. 18 BARS .
- SHELLS & FOLDED PLATE MEMBERS: NO. 6 BARS AND LARGER . NO. 5 BAR, W31 OR D31 WIRE AND SMALLER . .
- REINFORCING STEEL IN CONCRETE SHALL HAVE A MINIMUM LAP AS SCHEDULED, UNLESS OTHERWISE NOTED AND SHALL CONFORM TO ASTM A615, GRADE 60.
- ALL EXPOSED SURFACES OF CONCRETE SLABS, BEAMS AND COLUMNS EXPOSED TO THE EXTERIOR SHALL RECEIVE A SEALER IN CONFORMANCE TO THE SPECIFICATIONS.
- SLABS ON GROUND FOR INTERIOR CONDITIONED SPACES SHALL BE 4" THICK WITH 6x6 W2.1xW2.1 WWF PLACED 1 1/2" FROM TOP SURFACE OR WITH MACRO SYNTHETIC FIBERS AT 3 LBS PER YARD, PLACED OVER
- IØ MIL VAPOR BARRIER AND TERMITE TREATED COMPACTED SOIL, UNO. CONTRACTOR SHALL SAW CUT (1" DEEP \times 1/8" WIDE) CONTROL JOINTS IN SLABS ON GROUND WITHIN 24 HOURS AFTER PLACING CONCRETE, CONTROL JOINTS SHALL BE LOCATED AT CENTERS OF COLUMNS, UNDER
- NON-LOAD BEARING WALLS AND AT SPACINGS NOT GREATER THAN 15'-0" IN BOTH DIRECTIONS. PROVIDE DOWELS FOR LAP OF ALL HORIZONTAL REINFORCING BARS AT CORNERS AND TEES FOR WALL FOOTINGS, BEAMS, WALLS AND TIE BEAMS TO MAINTAIN CONTINUOUS REINFORCING THROUGH INTERSECTION.
- BEAMS (NOTED "CB-X") SHALL BE FORMED AND PLACED AFTER CONSTRUCTION OF SUPPORTING MASONRY WALLS OR COLUMNS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND CONSTRUCTION OF FORMWORK, BRACING AND SHORING IN CONFORMANCE WITH ACI 347-04 AND ACI 347R-05 10. TROWEL FINISHED SLAB-ON-GRADE SHALL HAVE THE FOLLOWING FLATNESS AND LEVELNESS VALUES:
- OVERALL = FF 38/ FL 25 ± LOCAL = FF 25/ FL 20 AS EXPRESSED IN ACI IIT, SECTION 4, AND MEASURED IN ACCORDANCE WITH ASTM E1155. PARTIALLY HARDENED CONCRETE, CONTAMINATED CONCRETE, RE-TEMPERED CONCRETE AND CONCRETE
- THAT HAS BEEN RE-MIXED AFTER IT HAS TAKEN ITS INITIAL SET ARE NOT TO BE USED IN NEW CONCRETE
- CONCRETE SHOULD BE CARRIED OUT AT SUCH A RATE THAT IT IS PLASTIC AT ALL TIME. 13. ALL REINFORCEMENT SHALL BE SECURELY HELD N PLACE WITH STANDARD ACCESSORIES WHILE PLACING CONCRETE. IF REQUIRED, ADDITIONAL BARS OR STIRRUPS SHALL BE PROVIDED BY CONTRACTOR AS

REINFORCING STEEL:

- REINFORCING STEEL FOR REINFORCED CONCRETE AND REINFORCED MASONRY SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED REINFORCEMENT, UNLESS OTHERWISE NOTED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185
- SPLICES SHALL BE IN CONFORMANCE WITH ACI 318-14 UNLESS OTHERWISE NOTED ELEVATED SLABS AND SLABS ON GROUND, EXCEPT SIDEWALKS, SHALL USE SHEET STOCK WELDED WIRE
- FABRIC. ROLL STOCK SHALL NOT BE USED FOR THESE MEMBERS. MECHANICAL SPLICING DEVICES FOR REINFORCING STEEL MAY BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR APPROVAL.

<u>FORMWORK AND SHORING:</u> NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 341 AND 301.

CONSTRUCTION JOINTS:

ANY DEVIATION OR ADDITION OF CONSTRUCTION JOINT FROM THAT SHOWN ON THE PLANS MUST BE REVIEWED BY THE ENGINEER. ALTERNATE OR ADDED CONSTRUCTION JOINT LOCATIONS ARE ACCEPTABLE ONLY AS A CHANGE ORDER, WHICH WILL INCLUDE ENGINEERING CHANGES BY THE ENGINEER OF RECORD FOR REDESIGN OF THE STRUCTURE, SHORING, ETC.

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

POST-INSTALLED ANCHORS:

- POST INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED
- ANCHORS FOR MISSING OR MISPLACED CAST-IN ANCHORS. CARE SHALL BE GIVEN TO AVOID DAMAGING EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE
- DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION
- INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCE AND/OR SPACING INDICATED IN THE MANUFACTURER'S LITERATURE. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL WITH CALCULATIONS PREPARED, SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE LOCALITY OF THE PROJECT SHOWING THAT THE
- SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN ACCEPTABLE PRODUCTS ARE:

 A) EXPANSION ANCHORS FOR NON-CRACKED CONCRETE ONLY:
- -WEDGE-ALL (WA) BY SIMPSON STRONG-TIE -KWIK BOLT 3 BY HILTI
- B) CRACKED CONCRETE MECHANICAL ANCHORS: -STRONG-BOLT (STB) BY SIMPSON STRONG-TIE -KWIK BOLT (TZ) BY HILTI
- C) SCREW ANCHORS: -TITEN HD (THD) BY SIMPSON STRONG-TIE
- -HUS-H BY HILTI D) ADHESIVE ANCHORS:
- FOR ANCHORING INTO SOLID BASE MATERIAL: -ACRYLIC -TIE (AT)
- -SET EPOXY-TIE (SET) WITH RETROFIT BOLTS (RFB) BY SIMPSON -HIT RE 500 BY HILTI
- 8. FOR ANCHORING INTO HOLLOW BASE MATERIAL: -CONTACT ENGINEER

CHEMICAL (ADHESIVE) ANCHORS:

6. SHALL BE AN EQUAL TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS RAMSET "EPCON", POWERS RAWL "POWER-FAST" CARTRIDGE SYSTEM, DUR-O-WAL "DUR-O-PAIR" EPOXY ANCHOR, OR HILTI HSE2421 EPOXY DOWELING SYSTEM, OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S

- METAL PLATE CONNECTED WOOD TRUSSES

 1. THE APPORTIONMENT OF THE RESPONSIBILITIES BETWEEN THE STRUCTURAL ENGINEER OF RECORD AND THE DELEGATED ENGINEER FOR WOOD TRUSSES SHALL BE AS SET FORTH IN CHAPTER 2 OF ANSI/TPI
- 1-2014, "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD CONSTRUCTION" WITH THE AMENDED DEFINITIONS SUBSTITUTED AS FOLLOWS: A. ANSI/TPI "TRUSS DESIGNER" REFERS TO THE DELEGATED ENGINEER FOR WOOD TRUSSES (TRUSS ENGINEER).
- B. ANSI/TPI "BUILDING DESIGNER" REFERS TO THE STRUCTURAL ENGINEER OF RECORD FOR METAL PLATE CONNECTED WOOD TRUSSES AND THEIR CONNECTIONS SHALL BE DESIGNED BY A DELEGATED ENGINEER FOR THE LOADS AND DESIGN CRITERIA PROVIDED HEREIN IN ACCORDANCE
- WITH THE EDITION OF THE LOCAL BUILDING CODE AND NDS REFERENCED UNDER GENERAL NOTES ON THIS SHEET AND SECTION 61G15-31.003 OF THE F.A.C. AS WELL AS 61G15-31.003. SUBMIT DELEGATED ENGINEERING DOCUMENTS FOR REVIEW AND DO NOT FABRICATE WITHOUT RECEIVING FINAL REVIEW. THE DELEGATED ENGINEER SHALL BE DEEMED A "TRUSS SYSTEM ENGINEER" AS DEFINED IN 6/GIB-31.003(4) OF F.A.C. AND SHALL DESIGN A TRUSS SYSTEM, MEANING THE ASSEMBLAGE OF TRUSSES AND TRUSS GIRDERS TOGETHER WITH ALL BRACING, CONNECTIONS AND OTHER STRUCTURAL ELEMENTS
- AND ALL SPACING AND LOCATIONAL CRITERIA THAT, IN COMBINATION, FUNCTION TO SUPPORT THE DEAD LOAD, LIVE LOAD, LIVE ROOF LOAD, AND WIND LOADS APPLICABLE TO THE TRUSS SYSTEM. SUPPORTING WALLS, FOUNDATIONS AND HEADERS ARE BEYOND THE SCOPE OF OTHER TRUSS SYSTEM ENGINEER'S SERVICES.
- 4. THE TRUSSES SHALL BE DESIGNED TO ACCOMMODATE THE SUPERIMPOSED LOADS AS TABULATED OR
- SPECIFIED HEREIN IN ADDITION TO THE TRUSS SELF-WEIGHT
- 5. THE TRUSS SHALL BE DESIGNED TO MEET THE SERVICEABILITY LIMITS AS FOLLOWS:

TRUSS CASE	LONG TERM CREEP	SHORT TERM CREEP	DEFLECTI
LOCATION	WITH A 1.5 FACTOR	WITH NO FACTOR	LIMIT
ROOF Lr	-	100% LIVE LOAD	L/360

- . TRUSS DESIGN DRAWINGS AND TRUSS BEARING REACTION VALUES & LOCATIONS, PREPARED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED, SHALL BE PROVIDED AND REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PROJECT PRIOR TO FABRICATION. TRUSS DESIGN DRAWINGS SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE PROJECT SITE, AND SHALL INCLUDE, AT A MINIMUM, THE FOLLOWING:
 - A. BASIC DESIGN WIND SPEED AND RISK CATEGORY.
 - B. SLOPE AND/OR DEPTH, SPAN AND SPACING LOCATION OF ALL JOINTS.
- REQUIRED BEARING WIDTHS. DESIGN LOADS AS PER LOAD TABLE

ROOF D + Lr 100% DEAD + 25% LIVE

ADJUSTMENTS TO LUMBER AND JOINT CONNECTOR DESIGN VALUES FOR CONDITIONS OF USE.

75% LIVE LOAD

- G. EACH REACTION FORCE, DIRECTION AND LOCATION. H. LUMBER SIZES, SPECIES AND GRADE FOR EACH MEMBER.
- JOINT CONNECTOR TYPE AND DESCRIPTION, E.G., SIZE THICKNESS OR GAUGE, AND THE DIMENSIONED LOCATION OF EACH JOINT CONNECTOR EXCEPT WHERE SYMMETRICALLY
- LOCATED RELATIVE TO THE JOINT INTERFACE. J. CONNECTIONS REQUIREMENTS FOR:
- (1) TRUSS-TO-TRUSS GIRDER, (2) TRUSS PLY-TO-PLY AND,
- (3) FIELD SPLICES. CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DESCRIPTION FOR LIVE AND TOTAL LOAD. . THE SIZE, CONNECTION AND LOCATION OF ALL TEMPORARY AND PERMANENT BRACING. 1. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER BUILDING IS FULLY
- COMPLETED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND ENSURE THE SAFETY OF WORKERS, THE BUILDING AND IT'S COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, ETC. THAT MAY BE NECESSARY. OBSERVATION VISITS BY THE ARCH/ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.
- 8. WHEN INSTALLING CONNECTORS ON PLATED TRUSSES (ON THE OPPOSITE SIDE OF THE TRUSS PLATE), DO NOT FASTEN THROUGH THE TRUSS PLATE FROM BEHIND, THIS MAY FORCE THE TRUSS PLATE OFF OF THE TRUSS AND COMPROMISE TRUSS PERFORMANCE.
- THE DELEGATED ENGINEER SHALL VERIFY THE APPLICABILITY OF THE SPECIFIED TRUSS CONNECTORS REGARDING THE ABILITY TO WITHSTAND UPLIFT, BEARING AND LATERAL LOADS AS MAY BE APPLICABLE AND MAKE RECOMMENDATIONS FOR SUBSTITUTE CONNECTORS AS REQUIRED.

1. STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL BE SURFACED DRY, USED AT 19% MAXIMUM MOISTURE CONTENT AND HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES OF NO. 2 SOUTHERN PINE GRADED AND STAMPED IN ACCORDANCE WITH THE SOUTHERN PINE INSPECTION BUREAU (SPIB) CONFORMING TO THE LATEST EDITION OF NDS, AS FOLLOWS:

SHEAR Fv = 175 psi. BENDING 2x6 Fb = 1,000 psi. BENDING 2x8 Fb = 925 psi. BENDING 2x10 Fb = 800 psi.

- BENDING 2x12 Fb = 750 psi. 2. WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH "AMERICAN WOOD-PRESERVES" ASSOCIATION STANDARDS. MEMBER SIZES SHOWN ARE NORMAL UNLESS NOTED OTHERWISE.
- 3. METAL CONNECTORS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE GALVANIZED IN CONFORMANCE WITH A6TM A653. NAILS IN PRESSURE TREATED LUMBER SHALL BE HOT DIPPED GALVANIZED TO CONFORM WITH ASTM A153 CLASS D. ELECTRO-GALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM SHALL NOT BE IN CONTACT WITH ACQ TREATED LUMBER.
- 4. THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED: 0.90
 - DEAD LOAD DEAD LOAD + FLOOR LIVE LOAD
- DEAD LOAD + ROOF LIVE LOAD 5. ROOF SHEATHING IS DESIGNED AS A DIAGRAM AND SHALL COMPLY WITH CHAPTER 23 OF THE FLORIDA BUILDING CODE, UN.O., SPAN RATED PANELS SHALL BE FASTENED TO NOMINAL 2X FRAMING SPACED AT 24" O.C. MAX. AS FOLLOWS:
- PANELS UP TO 1/2" THICK: 8D RING SHANK NAILS AT 6" O.C. EDGE, 12" O.C. ELSEWHERE PANELS UP TO 5/8" THICK: IØD RING SHANK NAILS AT 6" O.C. EDGE, 12" O.C. ELSEWHERE PANELS UP TO 3/4" THICK: 12D RING SHANK NAILS AT 6" O.C. EDGE, 12" O.C. ELSEWHERE
- 6. SEE ROOF SHEATHING NAILING PLAN FOR OTHER NAILING PATTERNS. 1. SPAN DIRECTION OF PLYWOOD SHEETS IS CONTRACTOR'S OPTION UNLESS SPECIFICALLY NOTED

STRUCTURAL PLAN INTENT IS TO SHOW STRUCTURAL MEMBERS AND FRAMING REQUIREMENTS ONLY. COORDINATE ALL DIMENSIONS, SIZES & LOCATIONS OF FLOOR, WALL, & CEILING OPENINGS & PENETRATIONS, AND ELEVATIONS WITH ALL DISCIPLINES FIELD CONDITIONS & GENERAL NOTES. NOTIFY ARCHITECT OF ANY DISCREPANCIES BEFORE STARTING ANY CONSTRUCTION, TYP, UON.

ADDITIONAL STRUCTURAL ABBREVIATIONS (SEE DRAWING GOOI FOR GENERAL ABBREVIATIONS) SEE AISC "STEEL CONSTRUCTION MANUAL" FOR STRUCTURAL STEEL NOTATIONS

BRG	BEARING	LL	LIVE LOAD
BB	BOND BEAM	MAS	MASONRY
BF	BOTH FACES	MTL	METAL
BN#W	BOLT, NUT & WASHER	MAX	MAXIMUM
B.O.	BOTTOM OF	NOM.	NOMINAL
BRDG	BRIDGING	OMEN	OPEN WEB STEEL JOIST
BS			PENETRATION
C=	CAMBER =	PC	PILE CAP
CFMT	COLD FORMED METAL TRUSS	PCL	PRE-CAST LINTEL
	CAST IN PLACE	PL	PLATE
	CLEAR		PREFABRICATED
	CONTINUOUS	PSL	PRE-STRESSED LINTEL
COL	COLUMN	PENET	PENETRATION
COORD	COORDINATION	R/RA	
DL	DEAD LOAD		REINFORCEMENT/ REINFORCING
DWL	DOWEL	SB	SLAB BEAM
DWG'S	DRAWINGS		SCHEDULE
EA	EACH	SHGWT	SEASONAL HIGH GROUND WATER TABLE
EE	EACH END	SW	SHEAR WALL
EF	EACH FACE	TB	TIE BEAM
ELEY.	ELEVATION	TE-1	THICKENED SLAB EDGE TYPE I
ES	EACH SIDE	T#G	TONGUE & GROOVE
ENGR	ENGINEER	THRD	THREAD (ED)
EXP	EXPANSION	T.O.	TOP OF
F-2	FOOTING TYPE 2	T.O.M.	TOP OF MASONRY
FTG	FOOTING	TOBP	TOP OF BEARING PLATE
G.T.	GIRDER TRUSS	TOPC	TOP OF PILE CAP
H.J.	HIPJACK	TOS	TOP OF STEEL
J.T.	JACKTRUSS	WB	WOOD BEAM

₩₽ ₩₩F

WALL FOOTING TYPE 1

WELDED WIRE FABRIC

IUORK POINT

ORGANIZATIONS & AGENCIES

HOLLOW CORE

KIPS (1000 LBS)

AMERICAN NATIONAL STANDARDS INSTITUTE AMERICAN SOCIETY OF CIVIL ENGINEERS
AMERICAN SOCIETY OF TESTING & MATERIALS ASCE ALUMINUM ASSOCIATION, INC.

HOLLOW STRUCTURAL SECTION

AMERICAN CONCRETE INSTITUTE AMERICAN INSTITUTE OF ARCHITECTS AMERICAN INSTITUTE OF STEEL CONSTRUCTION AMERICAN IRON AND STEEL INSTITUTE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION

AMERICAN WOOD COUNCIL AMERICAN WELDING SOCIETY FLORIDA BUILDING CODE

INTERNATIONAL BUILDING CODE LIGHT GAGE STRUCTURAL ENGINEERS ASSOCIATION NATIONAL CONCRETE MASONRY ASSOCIATION NATIONAL FOREST & PAPER ASSOCIATION

PRE-CAST PRE-STRESSED CONCRETE INSTITUTE POST TENSIONING INSTITUTE STEEL DECK INSTITUTE STEEL JOIST INSTITUTE

THE MASONRY SOCIETY WOOD TRUSS COUNCIL OF AMERICA

	WIND DESIGN CRITER	DESIGN CRITERIA	
			ASCE/SEI 7-16 REFERENCE
1	RISK CATEGORY	II	
2	BASIC WIND SPEED (Y "ULT")	145 MPH	SECT 26.5.1 FIG. 26.5-1B
3	WIND DIRECTIONALITY FACTOR	0.85	TABLE 26.6.
4	SURFACE ROUGHNESS CATEGORY	"C"	SECT. 26.72
5	EXPOSURE CATEGORY	"C"	SECT. 26.73
6	TOPOGRAHPIC EFFECTS/ TOPOGRAPHIC FACTOR	1.00	SECT. 26.8
٦	ENCLOSURE CLASSIFICATION:		
8	INTERNAL PRESSURE COEFFICIENT (ASCE 7-10, FIGURE 6-5)		
9	CANOPY	0.00	

ARCHITECTURE ■ DEVELOPMENT License #AA-C002117 7901 4TH ST. NORTH, SUITE 200 ■ ST. PETERSBURG, FL 33702 727 894-4453 www.bessolo.com

ENGINEERS, INC License # EB-0007103 7901 4TH ST. NORTH, SUITE 200 ST. PETERSBURG, FL 33702 (727) 894-4668

PROFESSIONAL SEAL

Daniel James Coorey, P.E. State of Florida 77065

SIGNED AND SEALED AND THE SIGNATURE MUST BE



ISSUED DATE: ISSUED FOR: 05-15-20 CONSTRUCTION DOCUMENTS

REVISIONS NO: DATE: DESCRIPTION \ | 10-15-20 | OWNER'S REVIEW

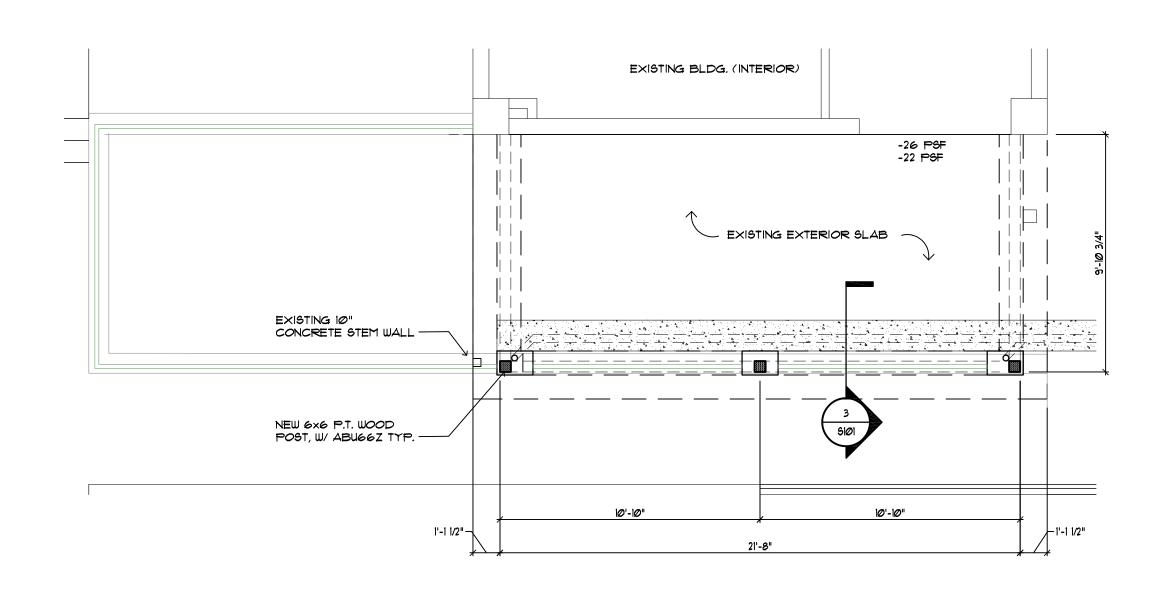
05-29-20

19079.00

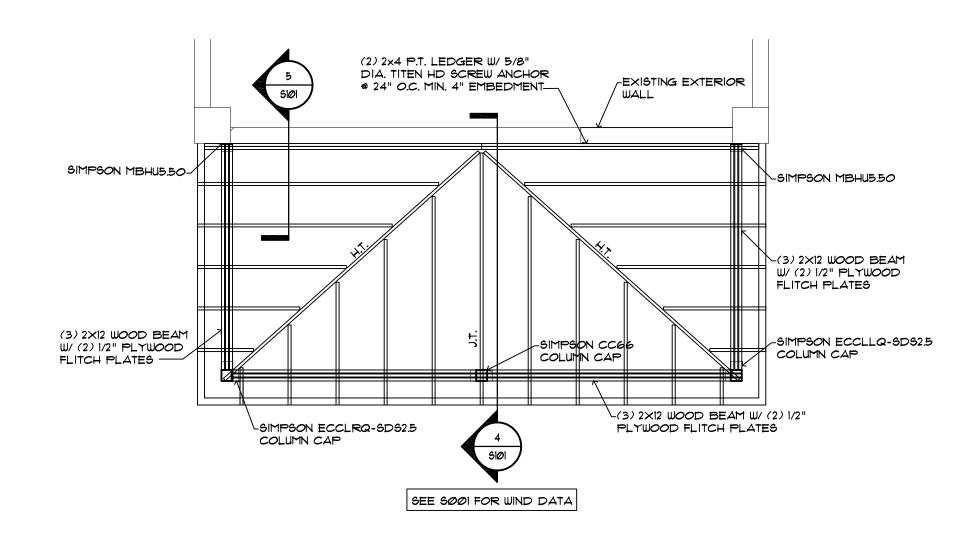
PROJECT NO:

PROJECT MANAGER:

STRUCTURAL GENERAL INFORMATION

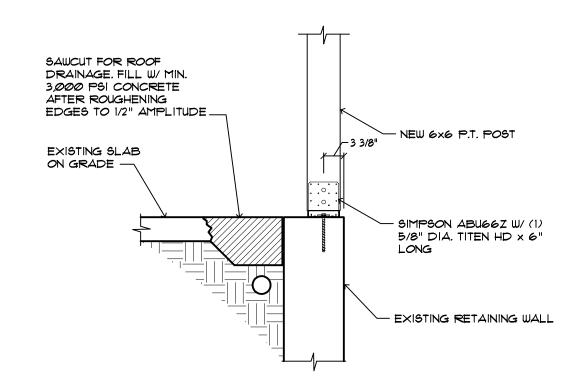






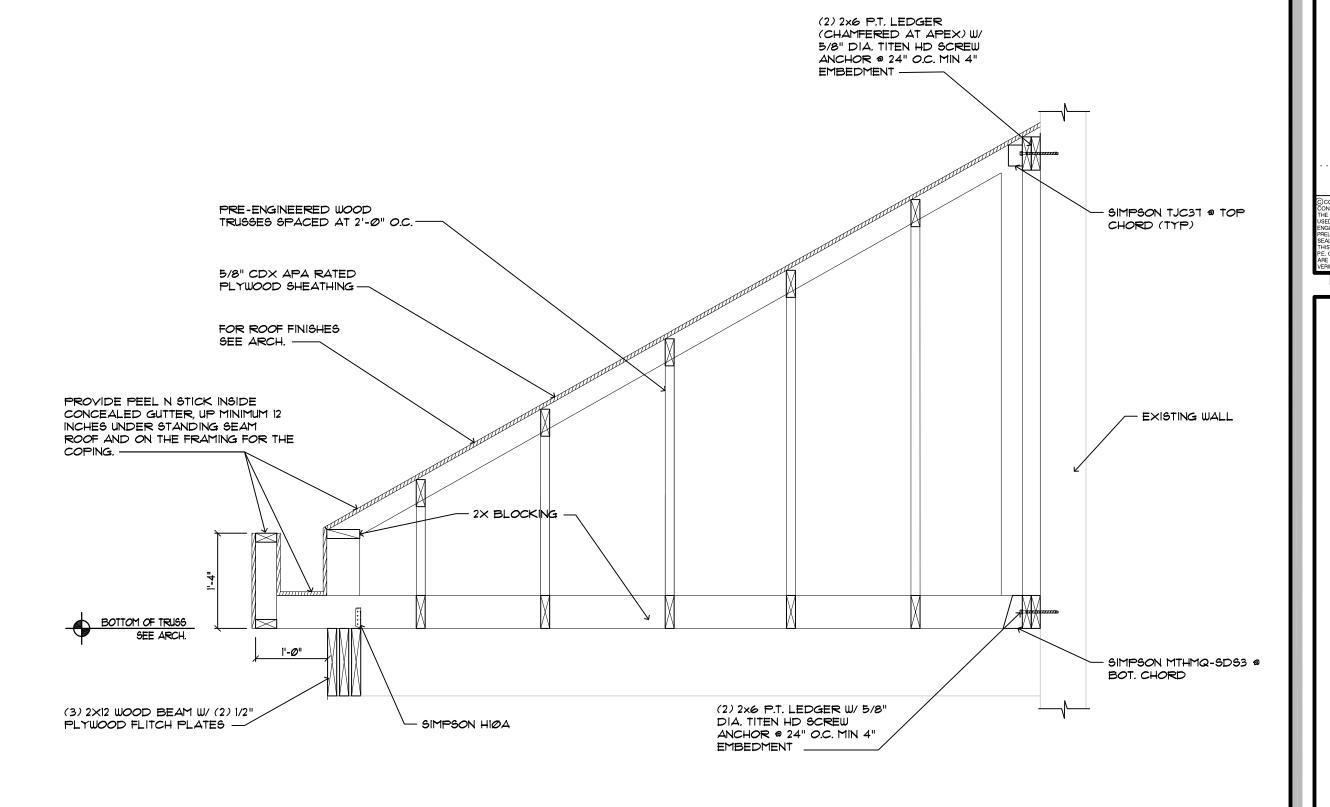
2 CANOPY FRAMING PLAN

9CALE: 1/4" = 1'-0"



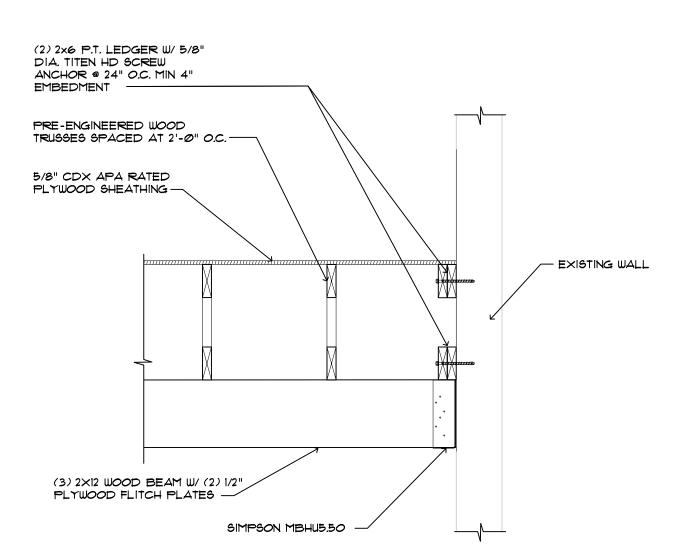
POST CONNECTION TO EXISTING STEM WALL

SCALE: 3/4" = 1'-0"



TRUSS BEARING ON WOOD BEAM

SCALE: 3/4" = 1'-0"



BEAM CONN. TO EXISTING WALL

SCALE: 3/4" = 1'-0"



KEM
ENGINEERS, INC
License # EB-0007103

License # EB-0007103
7901 4TH ST. NORTH, SUITE 200
ST. PETERSBURG, FL 33702
(727) 894-4668
PROFESSIONAL SEAL

Daniel James Coorey, P.E.
State of Florida 77065

RIGHT KEM ENGINEERS, INC. THIS DRAWING AND ALL INFORMATION NEED, INCLUDING ALL RELATED DIGITAL DRAWINGS OF FILES HERREIN ARE ELUSIVE PROPERTY OF KEM RONINEERS, INC. SHALL NOT BE COPIED OR ANY WAY WITHOUT THE EXPRESS WRITIEN CONSERT OF KEM RS, INC. AND MUST BE RETURNED UPON REQUEST. ALL DRAWINGS ARE VARY AND NOT FOR CONSTRUCTION PURPOSES UNLESS SIGNED AND BY DANIEL JAMES COOREY, THE DATE ADJACENT TO THE SEAL PRINTED COPIES OF THIS DOCUMENT THE DATE ADJACENT TO THE SEAL PRINTED COPIES OF THIS DOCUMENT.

DARBEE TOWERS
RENOVATION

100 E DRUID ROAD

ISSUED
DATE: ISSUED FOR:

	RFV	REVISIONS					
NO:	DATE:	DESCRIPTION					
Λ	10-15-20	OWNER'S REVIEW					

PROJECT NO: 19079.00

DRAWN BY: DJ

PROJECT MANAGER: RW

CHECKED BY: DJC

DATE: 05-29-20

SCALE: NOT TO SCALE

SIDI FOUNDATION, FRAMING PLANS AND DETAILS