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# DS101

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## FIRST DEMOLITION PLAN & ELEVATION



1. FLORIDA BUILDING CODE (ENHANCED) 2020
2. AISC 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE WITH COMPLEMENTARY STEEL - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES
3. AISC "SPECIFICATION FOR STEEL BUILDINGS"
4. AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"
5. AISC "SPECIFICATION FOR STRUCTURAL STEEL JOINTS USING ASTM A325 OR A430 BOLTS"
6. SJI "STANDARD SPECIFICATIONS FOR K SERIE, LH SERIE AND DLH SERIE OPEN WEB STEEL JOISTS AND FOR JOIST-TO-DECK DESIGN MANUAL" AND "ROOF DECK DESIGN MANUAL"
7. AISC "SPECIFICATION FOR STRUCTURAL STEEL JOINTS FOR MASONRY STRUCTURES" 15th 406/602-16
8. AISI "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS" AISI S320-12
9. AISI "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AISI S300-12
10. AISI "NATIONAL DESIGN SPECIFICATION FOR TIMBER CONSTRUCTION, 2010 EDITION"
11. AITC "TIMBER CONSTRUCTION MANUAL, 6TH EDITION, 2012"
12. ANSI/AISC SFDUP-1015 - SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC STANDARD
13. ASCE 2015
14. ASCE 7-16

1. THE CONTRACTOR SHALL VERIFY THE LOCATION OF UTILITIES IN THE AREA OF THE CONSTRUCTION. THE CONTRACTOR SHALL REPLACE ANY UTILITIES DAMAGED DURING CONSTRUCTION TO THE SATISFACTION OF THE OWNER OF THE PROJECT. THE CONTRACTOR SHALL PROTECT ALL EXISTING BUILDINGS AND STRUCTURES ADJACENT TO THE CONSTRUCTION BY THE CONTRACTOR DURING CONSTRUCTION.
2. SHOP DRAWINGS AS REQUIRED BY THE CONTRACT DOCUMENTS SHALL BE SUBMITTED BY THE CONTRACTOR FOR APPROVAL OF THE ARCHITECT/ENGINEER. THE CONTRACTOR SHALL NOT PROCEED WITH CONSTRUCTION OF ANY ELEMENTS REQUIRING SHOP DRAWINGS SHALL NOT COMMENCE UNTIL THE SHOP DRAWINGS ARE REVIEWED BY THE ARCHITECT/ENGINEER.
3. THE ARCHITECT/ENGINEER SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS PRIOR TO CONSTRUCTION AND SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION.
4. LOCATIONS AND SIZES OF OPENINGS, SLEEVES AND ANCHORAGE FOR ELECTRICAL AND MECHANICAL EQUIPMENT SHOWN ON STRUCTURAL PLANS ARE APPROXIMATE. THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHORAGE PRIOR TO CONSTRUCTION.
5. ALL ELEVATIONS ARE BASED ON THE FINISHED FLOOR ELEVATION OF THE GROUND FLOOR TO BE 0'-0".
6. THE CONTRACTOR SHALL VERIFY THE LOCATION AND DEPTH OF ALL DEPRESSIONS IN CONCRETE SLABS, ALL DEPRESSIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL, AND SHALL APPLY TO SIMILAR CONDITIONS, BOTH FIELD AND JOINTS.
7. THIS STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER IT'S BEEN FULLY COMPLETED. IT'S THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE STRUCTURE DURING CONSTRUCTION. THE CONTRACTOR SHALL INCLUDE THE ADDITION OF WHATEVER ADDITIONAL SECONDARY BRACING, ETC. THAT MAY BE NECESSARY, OBSERVATION.
8. VISITS TO THE SITE BY THE ARCHITECT/ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.
9. THE USE OF ANY OTHER MEANS OF CONSTRUCTION OR MATERIALS NOT SPECIFIED SHALL BE THE RESPONSIBILITY OF THE INSPECTOR SOLELY. ACCORDING TO THE SPECIFICATION AS PART OF THESE CONSTRUCTION DOCUMENTS.

1. PROPOSED CHANGES OR SUBSTITUTIONS TO STRUCTURAL DETAILS OR PLANS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD (EOR) FOR REVIEW. SUBMITTALS SHALL CONTAIN FULL DOCUMENTATION OF CHANGES OR SUBSTITUTIONS WITH SUPPORTING, SEALED CALCULATIONS (WHERE APPLICABLE), THE REVIEW OF CHANGES AND SUBSTITUTIONS, RE-ANALYSIS AND/OR RE-DRAFTING TO INCORPORATE CHANGES OR SUBSTITUTIONS INTO CONTRACT DOCUMENTS ARE ADDITIONAL SERVICES FOR EOR. CONSTRUCTION COST REVISIONS ARE BETWEEN THE CONTRACTOR AND OWNER AND ARE NOT REVIEWED BY THE EOR.

UNLESS OTHERWISE NOTED, THE OWNER SHALL REQUIRE A CERTIFIED INDEPENDENT TESTING LABORATORY THAT IS NOT THE TESTING ENGINEER TO PERFORM ALL TESTS. INSPECTIONS AND SUBMITTALS OF THE RESULTS AS REQUIRED IN THE PROJECT SPECIFICATIONS THIS INCLUDES, BUT IS NOT LIMITED TO:

- 1. INSPECTION OF BEARING CAPACITY OF FOUNDATION SOILS.
  - INSPECTION OF COMPACTION OF FILL.
  - INSPECTION OF REINFORCED CONCRETE CONSTRUCTION.
  - INSPECTION OF STRUCTURAL MASONRY CONSTRUCTION.
  - INSPECTION OF STRUCTURAL STEEL CONSTRUCTION.
- 2. THE TESTING AND INSPECTIONS SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A QUALIFIED PROFESSIONAL STRUCTURAL ENGINEER CURRENTLY REGISTERED IN FLORIDA.
- 3. THE TESTING AND INSPECTIONS SHALL BE CERTIFIED BY THE TESTING ENGINEER TO THE STATE OF FLORIDA AS QUALIFIED TO PERFORM THE TESTS AND INSPECTIONS WHICH THEY PERFORM.

1. ANY DEVIATION OR ADDITION OF CONSTRUCTION JOINT FROM THAT SHOWN ON THE PLANS MUST BE REVIEWED BY THE ENGINEER. ALTERNATE OR ADDED CONSTRUCTION JOINT LOCATIONS ARE ACCEPTABLE ONLY AS A CHANGE ORDER, WHICH WILL INCLUDE ENGINEERING CHARGES BY THE ENGINEER OF RECORD FOR REDESIGN OF THE STRUCTURE, SHORING, ETC.

1. NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE LOCATED ON THESE DRAWINGS WITHOUT PREVIOUS APPROVAL OF THE ENGINEER.

1. SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS RAMSET "EPCON", POWERS RAWL "POWER-FAST" CARTRIDGE SYSTEM, DUR-O-WAL "DUR-O-PAIR" EPOXY ANCHOR, OR HILTI H62421 EPOXY DOWELING SYSTEM, OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE.

2. POST INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR ANY UNDESIGNED PLACED CAST-IN ANCHORS.
3. CARE SHALL BE GIVEN TO AVOID DAMAGING EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS.
4. UNLESS SPECIFICALLY NOTED OTHERWISE, ALL EMBEDDED ANCHORS IN THE APPROPRIATE SUBSTRATE, IN THE CASE OF MASONRY THE CONTRACTOR IS TO EMBED ANCHORS INTO FULLY GROUTED CELLS WITH A MINIMUM EMBEDMENT OF 8 TIMES THE NOMINAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD, AND TO MINIMIZE EMBEDMENT PER THE MANUFACTURER'S RECOMMENDATIONS. INSTRUCTIONS ARE NOT LESS THAN MINIMUM EDGE DISTANCE AND/OR SPACING INDICATED IN MANUFACTURER'S LITERATURE.
5. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL, WITH CALCULATIONS PREPARED, SIGNED AND SEALED BY AN ENGINEER REGISTERED IN FLORIDA OF THE PROJECT SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE.
6. ACCEPTABLE PRODUCTS ARE:
  - A) EXPANSION ANCHORS FOR NON-CRACKED CONCRETE ONLY:
    - WEDGE-ALL (WA) BY SIMPSON STRONG-TIE
    - KEWIK BOLT 3 BY HILTI
  - B) CRACKED CONCRETE MECHANICAL ANCHORS:
    - STRONG-BOLT (STB) BY SIMPSON STRONG-TIE
    - KEWIK BOLT (TZ) BY HILTI EBR-191
  - C) SCREW ANCHORS:
    - TITEN HD (THD) BY SIMPSON STRONG-TIE
    - KEWIK HUB BY HILTI
  - D) ADHESIVE ANCHORS:
7. FOR ANCHORING INTO SOLID BASE MATERIAL:
  - ACRYLIC TIE (AT)
  - SET EPOXY-TIE (SET) WITH RETROFIT BOLTS (RFB) BY SIMPSON
  - HIT RE 500 SD BY HILTI EBR-2322
8. FOR ANCHORING INTO HOLLOW BASE MATERIAL:
  - CONTRACT ENGINEER

1. ALL ROOF TRUSSES CONNECTOR SHALL BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW ACCORDING TO THE DELEGATED TRUSS ENGINEER. SEE TRUSS MANUFACTURER DRAWINGS FOR DETAILS. # ANCHORAGE REQUIREMENTS.
2. ALL EXTERIOR PRESSURE DIAGRAM ON ROOF. # DEFINITION OF 'g'. # NAILING DIAGRAM.
3. SEE DELEGATED ENGINEER SUBMITTAL ANCHOR RODS SIZES, DETAILS & REQUIREMENTS.
3. INTERIOR WALLS WITH ROOF TRUSS BEARING SHALL BE MECHANICALLY FASTENED AT ALL BEARING POINTS AND STUD FRAMING BELOW, ACCORDING TO THE SHEARWALL DIAGRAM # SPECS. # ANCHORAGE REQUIREMENTS.

2. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN ANCHORS.
3. CARE SHALL BE GIVEN TO AVOID DAMAGING EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER MANUFACTURER'S INSTRUCTIONS.
4. UNLESS SPECIFIED OTHERWISE, ANCHORS SHALL BE EMBEDDED IN THE APPROPRIATE SUBSTRATE WITH A MINIMUM OF EIGHT (8) TIMES THE NORMAL ANCHOR DIAMETER OR THE EMBEDMENT REQUIRED TO SUPPORT THE INTENDED LOAD. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN A MINIMUM EDGE DISTANCE AND/OR SPACING INDICATED IN THE MANUFACTURER'S LITERATURE.
5. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW SHALL BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO REMOVAL WITH CALCULATIONS PREPARED, SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE LOCALITY OF THE PROJECT SHOWING THAT THE SUBSTITUTED PRODUCT WILL ACHIEVE AN EQUIVALENT CAPACITY USING THE APPROPRIATE DESIGN PROCEDURE.
6. ACCEPTABLE PRODUCTS ARE:
  - A) EXPANSION ANCHORS FOR NON-CRACKED CONCRETE ONLY:
    - EDEGE-ALL (1/4") BY SIMPSON STRONG-TIE
    - KWIK BOLT 3 BY HILTI
  - B) CRACKED CONCRETE MECHANICAL ANCHORS:
    - STRONG-BOLT (9TB) BY SIMPSON STRONG-TIE
    - KWIK BOLT (TZ) BY HILTI
  - C) BOREFL ANCHORS:
    - TITEN HD (THD) BY SIMPSON STRONG-TIE
    - HUS-H BY HILTI
  - D) ADHESIVE ANCHORS:
    - FOR ANCHORING INTO SOLID BASE MATERIAL:
      - ACRYLIC -TIE (AT)
      - SET EPOXY-TIE (SET)
      - HIT RE 500 BY HILTI
    - FOR ANCHORING INTO MOLLOW BASE MATERIAL:
      - CONTRACT ENGINEER

SHALL BE AN EQUAL TWO PART EPOXY FOLYMER INJECTION SYSTEM, SUCH AS KAMSET EPCON, POWERS RAWL "POWER-FAST" CARTRIDGE SYSTEM, DUR-O-WAL "DUR-O-PAIR" EPOXY ANCHOR, OR HILTI HSE2421 EPOXY DOUINGL SYSTEM, OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE.

1. THE DELEGATED ENGINEER FOR WOOD TRUSSES SHALL BE AS SET FORTH IN CHAPTER 2 OF ANSI/APA-1204, "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION" WITH THE AMENDED DEFINITIONS SUBSTITUTED AS FOLLOWS:
  - A. ANSI/APA TRUSS DESIGNER: REFERS TO THE DELEGATED ENGINEER FOR WOOD TRUSSES (TRUSS ENGINEER).
  - B. ANSI/APA "BUILDING DESIGNER" REFERS TO THE STRUCTURAL ENGINEER OF RECORD FOR THE PROJECT.
2. METAL PLATE CONNECTED WOOD TRUSSES AND THEIR CONNECTIONS SHALL BE DESIGNED BY A DELEGATED ENGINEER FOR WOOD TRUSSES IN ACCORDANCE WITH THE REQUIREMENTS OF THE NATIONAL DESIGN STANDARD AND NDS REFERENCED UNDER GENERAL NOTES ON THIS SHEET AND SECTION 6.01B-31.003 OF THE F.A.C. AS WELL AS 6.01B-31.003, SUBMIT DELEGATED ENGINEERING DOCUMENTS FOR REVIEW AND DO NOT FABRICATE WITHOUT RECEIVING FINAL REVIEW.
3. THE TRUSS SYSTEM ENGINEER SHALL BE THE TRUSS SYSTEM ENGINEER" AS DEFINED IN 6.01B-31.003(2) OF THE F.A.C. AND SHALL DESIGN A TRUSS SYSTEM MEANING THE ASSEMBLY OF TRUSSES AND TRUSS GIRDERS TOGETHER WITH ALL BRACING, CONNECTIONS AND OTHER STRUCTURAL ELEMENTS AND ALL SPACING AND SUBSTITUTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE NATIONAL DESIGN STANDARD, LOAD, AND WIND LOADS APPLICABLE TO THE TRUSS SYSTEM, SUPPORTING WALLS, FOUNDATIONS AND HEADERS ARE BEYOND THE SCOPE OF OTHER TRUSS SYSTEM ENGINEER'S SERVICES.
4. THE TRUSSES SHALL BE DESIGNED TO ACCOMMODATE THE SUPERIMPOSED LOADS AS TABULATED OR SPECIFIED HEREIN IN ADDITION TO THE TRUSS SELF-WEIGHT.
5. THE TRUSS SHALL BE DESIGNED TO MEET THE SERVICEABILITY LIMITS AS FOLLOWS:

TRUSS CASE LOCATION	LONG TERM CREEP WITH A 1.5 FACTOR	SHORT TERM CREEP WITH NO FACTOR	DEFLECTION LIMIT
ROOF Lr	-	100% LIVE LOAD	L/360
ROOF D + Lr	100% DEAD + 25% LIVE	75% LIVE LOAD	L/240

6. TRUSS DESIGN DRAWINGS AND TRUSS BEARING REACTION VALUES & LOCATIONS, PREPARED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED, SHALL BE PROVIDED AND REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PROJECT PRIOR TO FABRICATION. TRUSS DESIGN DRAWINGS SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE PROJECT SITE, AND

SHALL INCLUDE, AT A MINIMUM, THE FOLLOWING:

- A. BASIC DESIGN WIND SPEED AND RISK CATEGORY.
- B. SLOPE AND/OR DEPTH, SPAN AND SPACING

D. REQUIRED BEARING WIDTHS.  
E. DESIGN LOADS AS PER LOAD TABLE  
F. ADJUSTMENTS TO LUMBER AND JOINT CONNECTOR DESIGN VALUES FOR CONDITIONS OF USE.  
G. EACH REACTION FORCE, DIRECTION AND LOCATION

1. JOINT CONNECTOR TYPE AND DESCRIPTION, E.G., SIZE THICKNESS OR GAUGE, AND THE DIMENSIONED LOCATION OF EACH JOINT CONNECTOR EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE.

K. CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DESCRIPTION FOR LIVE AND TOTAL LOAD.  
L. THE SIZE, CONNECTION AND LOCATION OF ALL TEMPORARY AND PERMANENT BRACING.

1. THIS STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND ENSURE THE SAFETY OF WORKER, THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADEQUATE WEIGHTS AND BRACING TO BE MAINTAINED THROUGHOUT THE ERECTION. NECESSARY OBSERVATION VISITS BY THE ARCH/ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.
2. WHEN INSTALLING CONNECTORS ON PLATED TRUSSES (ON THE OPPOSITE SIDE OF THE TRUSS PLATE) DO NOT REMOVE THE TRUSS PLATE FROM BEHIND. THIS MAY FORCE THE TRUSS PLATE OFF THE TRUSS AND COMPROMISE TRUSS PERFORMANCE.
3. THE DELEGATED ENGINEER SHALL VERIFY THE APPLICABILITY OF THE SPECIFIED TRUSS CONNECTORS TO THE TRUSS AND THE APPLIED LOADS AS THEY BE APPLICABLE AND MAKE RECOMMENDATIONS FOR SUBSTITUTE CONNECTORS AS REQUIRED.

CONNECTORS SHALL BE GALVANIZED (Z-MAX COATED). CONNECTOR MODEL NUMBERS SHOWN ARE STRONG TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG TIE CO., 1450 DOOLITTLE DR., P.O. BOX 1568, SAN LEANDRO, CA 94571. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. UNLESS SHOWN OTHERWISE, INSTALL SIZE AND NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG.

ROOF TRUSS SYSTEMS, LIGHT GAGE STEEL EXTERIOR WALL SYSTEMS, ALUMINUM SYSTEMS, GLAZED CURTAIN WALLS, PREFABRICATED STEEL STAIRS & RAILINGS, STRUCTURAL STEEL CONNECTIONS REQUIRING ENGINEERING PREFABRICATED BAY WINDOW FRAMING FOR UNITS.

2. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND PLANS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. CALCULATIONS AND SHOP DRAWINGS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.

3. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER

4. SHOP DRAWINGS AND CALCULATIONS REQUIRE THE IMPRESSED SEAL, DATE AND SIGNATURE OF THE DELEGATED ENGINEER. COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BEAR THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. SEFAS DO NOT REQUIRE THE IMPRESSED SEAL AND SIGNATURE OF THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED BLUELINE PRINT FOR RECORD.

5. DRAWINGS PREPARED SOLELY TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

6. CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.

7. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:

B. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.

C. THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. (NO DETAILED CHECK OF CALCULATIONS WILL BE MADE.)

D. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. (NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE). SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

1	INSPECTION OF FIELD GLUING OPERATIONS OF ELEMENTS OF THE MAIN WINDFORCE RESISTING SYSTEM.	FIELD INSPECTION	CONTINUOUS
2	INSPECTION OF NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE MAIN WINDFORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWNS.	SHOP (3) AND FIELD INSPECTION	PERIODIC

1.	ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.	SHOP (3) AND FIELD INSPECTION	PERIODIC
2.	EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS.	SHOP (3) AND FIELD INSPECTION	PERIODIC

	RISK CATEGORY	II	ASCE/SEI 7-16 REFERENCE
1	BASIC WIND SPEED (V "ULT")	145 MPH	SECT 26.5.1 FIG. 26.5-1B
3	WIND DIRECTIONALITY FACTOR	0.85	TABLE 26.6-1
4	SURFACE ROUGHNESS CATEGORY	"C"	SECT. 26.7-2
5	EXPOSURE CATEGORY	"C"	SECT. 26.7-3
6	TOPOGRAPHIC EFFECTS/ TOPOGRAPHIC FACTOR	1.0	SECT. 26.8
7	ENCLOSURE CLASSIFICATION		
8	MAIN BUILDING	ENCLOSED	SECT. 26.11 & 26.2
9	FORTE COCHERE & CANOPIES	OPEN	SECT. 26.11 & 26.2
10	INTERNAL PRESSURE COEFFICIENT (ASCE 7-16, SECTION 16.13)		
10A	MAIN BUILDING	v= 0.18	
10B	FORTE COCHERE & CANOPIES	0.20	REF SHEET 50002
11	PRESSURES ON COMPONENTS & CLADDING		REF SHEET 50002
11A	SEE ROOF PRESSURE DIAGRAM FOR ROOF		REF SHEET 50002A, B, C
11B	SEE WALL TABLE FOR WALLS		REF SHEET 50002A, B, C
12	WIND BORNE DEBRIS REGION	APPLIES	
	DIRECTIONAL PROCEDURES, HURRS	APPLIES	CHAPTER 21
	COMPONENTS & CLADDING PROCEDURES	APPLIES	CHAPTER 30
	BUILDINGS NOT SENSITIVE TO DYNAMIC EFFECTS	APPLIES	CHAPTER 26
	NO WIND CHANNELING OR BUFFETING EFFECTS	APPLIES	CHAPTER 26
	CLADDING AS SIMPLIFIED DIAPHRAGM BUILDING - PER ASCE-7 SECTION 16.2	APPLIES	CHAPTER 26

OCCUPANCY OR USE		SQL	ILE
		20	40
		10	40
CORRIDORS	1ST FLOOR	20	100
		20	
DINING		20	100
LOBBIES		20	100
OFFICES		20	50
MECHANICAL ROOMS		20	60
STORAGE		20	15
PARTITIONS		0	15
		20	20
	FLAT	10	0
		BOTT. CHORD (COMMON AREAS NOT INCL. RESIDENTIAL CORRIDORS)	15
		TOP CHORD	20
		20	20
	SLOPED	BOTT. CHORD	0
		BOTT. CHORD (COMMON AREAS NOT INCL. RESIDENTIAL CORRIDORS)	10
		TOP CHORD	0

2. SDL: DENOTES SUPERIMPOSED DEAD LOAD.
3. LL : DENOTES LIVE LOAD

ADDITIONAL STRUCTURAL ABBREVIATIONS  
(SEE DRAWING G001 FOR GENERAL ABBREVIATIONS  
SEE AISC "STEEL CONSTRUCTION MANUAL" FOR  
STRUCTURAL STEEL NOTATIONS)

BBS	BEARING	LL	LIVE LOAD
BDB	BOND BEAM	MAS	MASONRY
BDF	BOTH FACES	MTL	METAL
BNO	BOLT, NUT & WASHER	MAX	MAXIMUM
BW	BOTTOM OF	NOM.	NOMINAL
BRGS	BRIDGES	OPEN	OPEN WEB STEEL JOIST
BSS	BOTH SIDES	PFENET	PENETRATION
C	CAMBER	PC	PILE CAP
CC	COL. JOINT	PRECAST	PRECAST LINTEL
CLIP	CAST IN PLACE	FL	FLATE
CIR	CLEAR	PREFAB	PREFABRICATED
CONT.	CONTINUOUS	PSL	PRESTRESSED LINTEL
COL	COLUMN	PFENET	PENETRATION
COORD	COORDINATION	R/R	RADIUS
DL	DEAD LOAD	REIN	REINFORCEMENT/ REINFORCING
DIL	DILUENT	SLAB	SLAB
DUGS	DRAWINGS	SCHED	SCHEDULE
EA	EACH	SHGWT	SEASONAL HIGH GROUND WATER TAB
EF	EACH END	SU	SHEAR LUG
EF	EACH FACE	TE	TE BEAM
ELEV.	ELEVATION	TE-1	THICKENED SLAB EDGE TYPE I
ES	ENGINEER	TG	TONGUE & GROOVE
ENGR	ENGINEER	THRD	THREAD (ED)
EXP	EXPANSION	T.O.	TOP OF
F	FOOTING TYPE 2	T.O.M.	TOP OF MASONRY
FG	FOOTING	T.O.BP	TOP OF BEARING PLATE
G.T.	GIRDER TRUSS	TOPC	TOP OF PILE CAP
H	HIFI JACK	TOS	TOP OF STEEL
JT	JACKTRUSS	UB	WOOD BEAM
HCS	HOLLOW CORE	UF-1	WALL FOOTING TYPE I
H8	HOLLOW STRUCTURAL SECTION	UF-2	WORK POINT
HS	HIPS (H202 LB3)	UF	WELDED WIRE FABRIC

ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS
AA	ALUMINUM ASSOCIATION, INC.
ACI	AMERICAN CONCRETE INSTITUTE
AI	AMERICAN INSTITUTE OF ARCHITECTS
ASCE	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
AUC	AMERICAN WOOD COUNCIL
AWS	AMERICAN WELDING SOCIETY
IBC	FLORIDA BUILDING CODE
IBC	INTERNATIONAL BUILDING CODE
LGSEA	LIGHT GAUGE STRUCTURAL ENGINEERS ASSOCIATION
NGMA	NATIONAL CONCRETE MASONRY ASSOCIATION
NFA	NATIONAL FOREST & PARKS ASSOCIATION
PCFI	PRE-CAST PRE-STRESSED CONCRETE INSTITUTE
PTI	POST TENSIONING INSTITUTE
SDI	STEEL DECK INSTITUTE
SJI	STEEL JOIST INSTITUTE
THS	THE MASONRY SOCIETY
WTCA	WOOD TRUSS COUNCIL OF AMERICA



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ORLANDO  
HOUSING AUTHORITY  
OHAT018 REEVES COURT  
1143/1145 E SOUTH STREET  
ORLANDO, FLORIDA 32801

DATE:	ISSUED FOR:
12-28-23	CONSTRUCTION DOCUMENTS

[illegible]

PROJECT NO: 2300200  
DRAWN BY: LMM  
PROJECT MANAGER: TSS  
CHECKED BY: KJB  
DATE: 12-28-23  
SCALE: AS NOTED

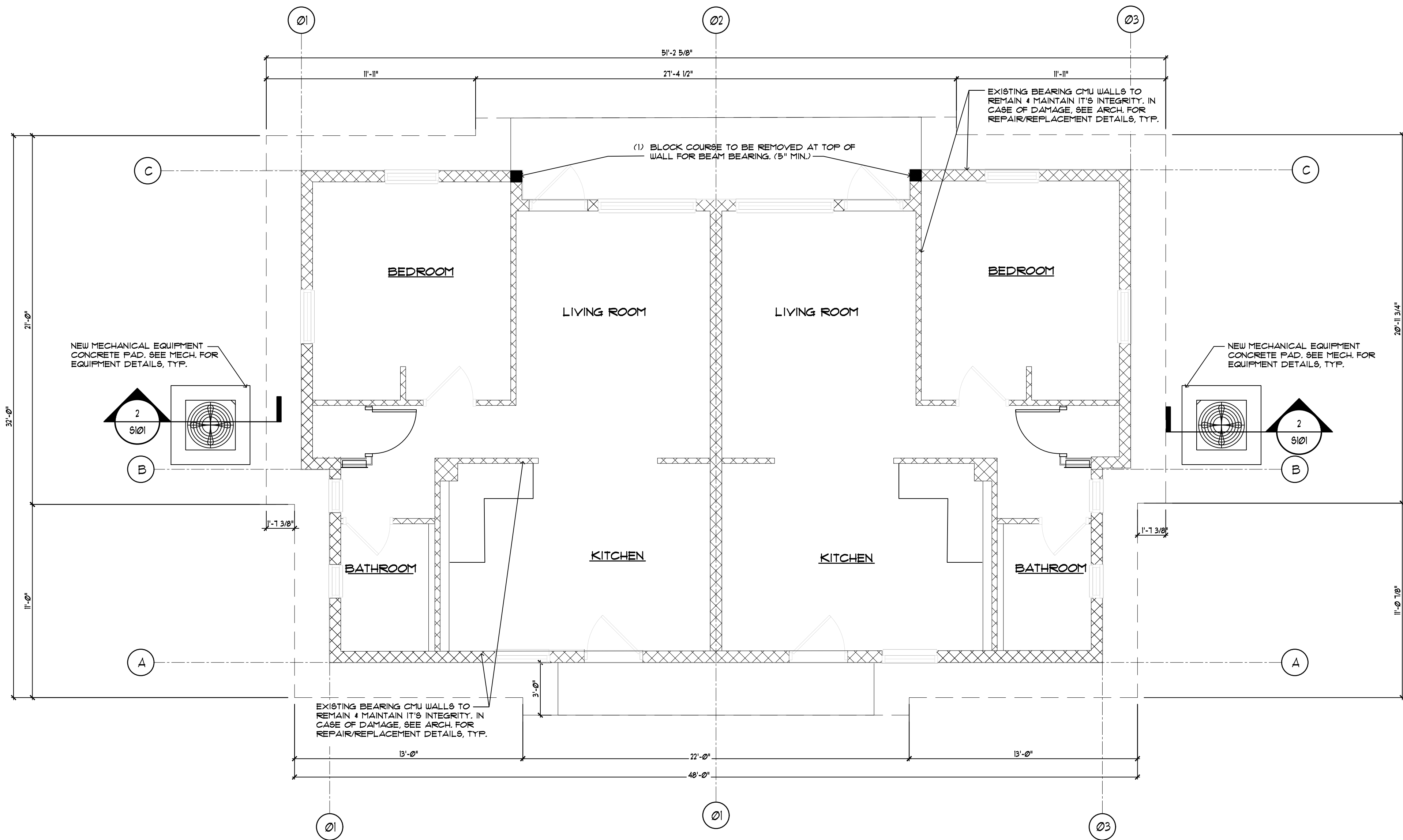
# S001

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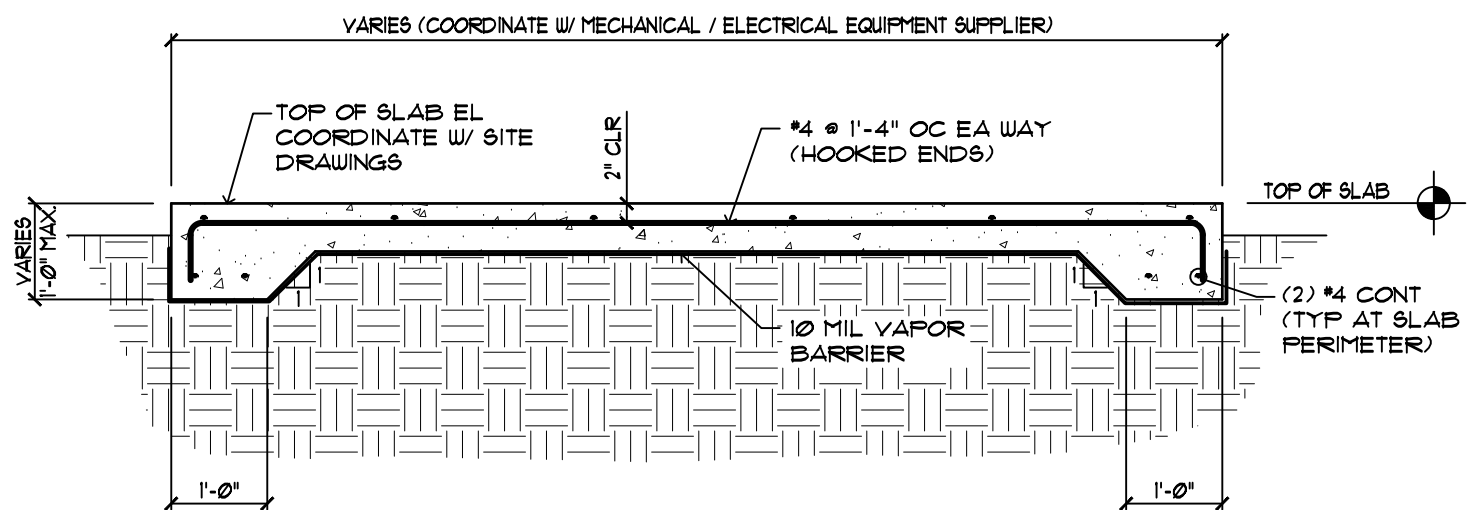
## GENERAL STRUCTURAL INFORMATION







1 FIRST FLOOR PLAN  
SCALE: 1/4" = 1'-0"



2 EXTERIOR MECHANICAL EQUIPMENT SLAB  
SCALE: 3/4" = 1'-0"

GENERAL NOTES

1. ALL EXISTING WORK TO REMAIN, WHICH IS DAMAGED OR MADE IMPERFECT, SHALL BE REPLACED OR REPAIRED TO THE ORIGINAL CONDITION, AS DETERMINED BY THE ARCHITECT.
2. VERIFY DIMENSIONS & ELEVATIONS WITH ARCH. DRAWINGS BEFORE COMMENCING CONSTRUCTION. FOR DIMENSIONS NOT SHOWN, SEE OTHER STRUCTURAL DRAWINGS AND VERIFY W/ ARCH. DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR THE STRUCTURAL ENGINEER.
3. SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB.
4. ELEVATIONS SHOWN ON PLAN AND DETAILS ARE IN REFERENCE TO EXISTING FLOOR & ROOF ELEVATIONS.
5. SEE S001 FOR GENERAL NOTES.

LEGEND:

EXISTING CMU WALL TO REMAIN

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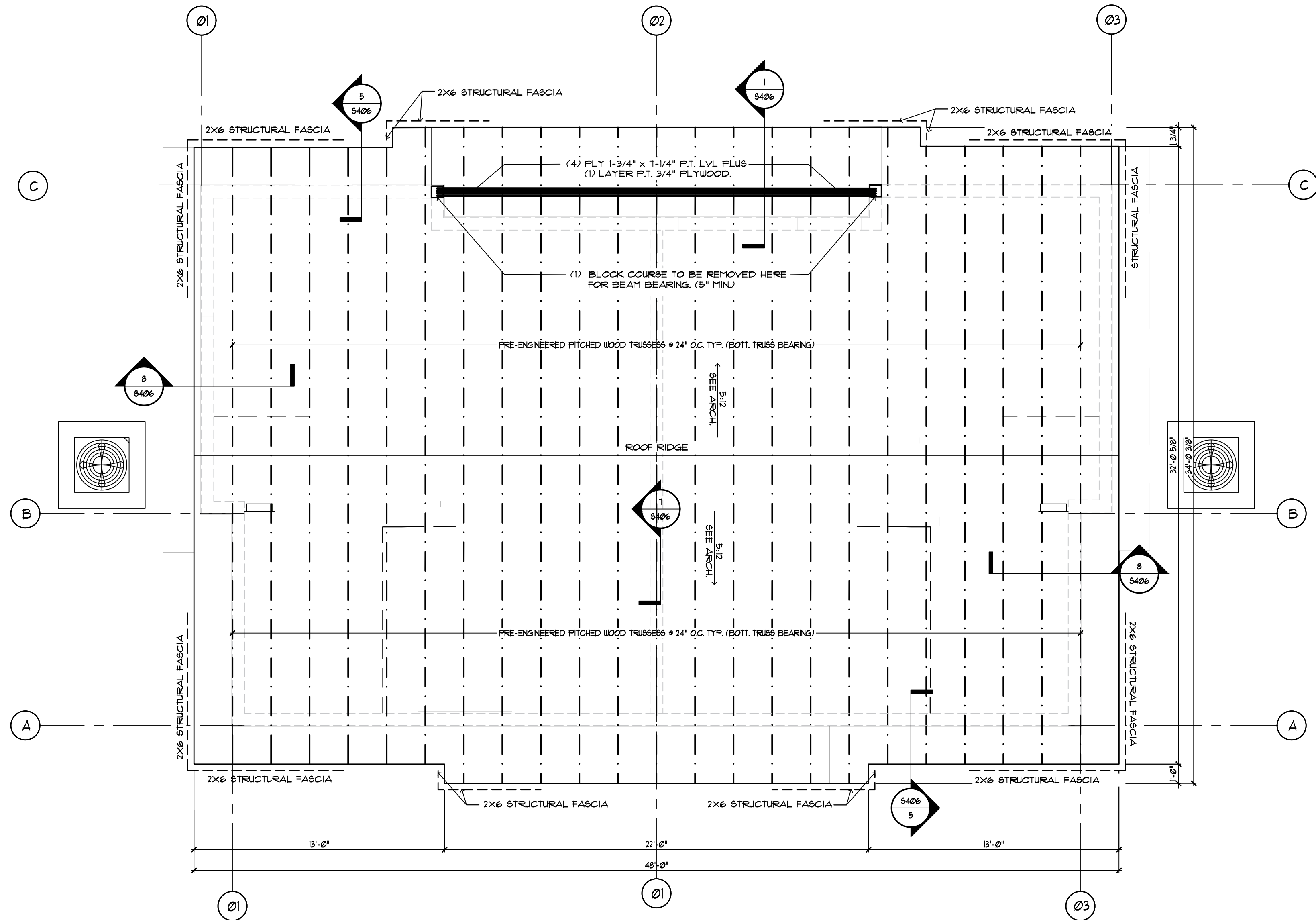
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NO.	DATE	DESCRIPTION

PROJECT NO.: 23001200  
DRAWN BY: LMM  
PROJECT MANAGER: T98  
CHECKED BY: KJB  
DATE: 12-28-23  
SCALE: AS NOTED

S101  
FIRST FLOOR PLAN & DETAILS





2 GABLE END ROOF FRAMING PLAN  
SCALE: 1/4" = 1'-0"

ROOF PLAN NOTES:

GENERAL:

1. VERIFY DIMENSIONS & ELEVATIONS WITH ARCH. DRAWINGS BEFORE COMMENCING CONSTRUCTION. FOR DIMENSIONS NOT SHOWN SEE OTHER STRUCTURAL DRAWINGS AND VERIFY W/ ARCH. DRAWINGS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR THE STRUCTURAL ENGINEER.
2. SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS AND DRAIN LOCATIONS ON ROOF PLANS.
3. ROOF CONSTRUCTION SHALL BE METAL PLATED WOOD TRUSSES IN THE FRAMING CONFIGURATION SHOWN ON THE PLANS.
4. ELEVATIONS SHOWN ON PLANS ARE IN REFERENCE TO TOP OF GROUND FLOOR ELEVATION 0'-0".
5. SEE S001 FOR GENERAL NOTES.
6. RCB-\* INDICATES A CONCRETE BEAM AT THE ROOF.
7. FOR SIZE & REINFORCEMENT, BEAMS SHALL BEAR ON WALLS A MIN. OF 8" EA. END. COORDINATE WITH BOND BEAM & Lintel LOCATIONS.
8. WRE-\* INDICATES A P.T. WOOD BEAM AT THE ROOF - SOUTHERN PINE NO. 1.
9. SC-\* INDICATES A STEEL COLUMN.
10. BEAMS AND ALL OTHER CONCRETE STRUCTURAL MEMBERS SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI MINIMUM.

WOOD TRUSSES:

1. SEE SHEET S001 FOR GENERAL WOOD TRUSS NOTES.
2. THE WOOD TRUSSES SHALL BE DESIGNED BY A SPECIALTY ENGINEER RETAINED BY THE TRUSS SUPPLIER. THE TRUSS ENGINEER SHALL BE RESPONSIBLE FOR ALL TRUSS MEMBER TO TRUSS MEMBER CONNECTIONS.
3. THE TRUSS ENGINEER SHALL SUBMIT, ALONG WITH THE SHOP DRAWINGS, THE TRUSS REACTIONS IMPOSED ONTO THE STRUCTURE.
4. THE ENGINEER OF RECORD WILL BE RESPONSIBLE FOR ALL TRUSS TO SUPPORTING STRUCTURE CONNECTIONS.
5. CN\* INDICATES A ROOF TRUSS CONNECTOR.
6. ALL TRUSSES USE CN-1 UNO.
7. THE TRUSS CONNECTORS MAY GET MODIFIED UPON REVIEW OF TRUSS REACTIONS SUBMITTED BY THE TRUSS ENGINEER.
8. TRUSS ENGINEER SHALL ADHERE TO THE FRAMING SHOWN ON PLANS AS MUCH AS POSSIBLE IN ORDER TO KEEP THE LOAD DISTRIBUTION TO THE SUPPORTING STRUCTURE UNCHANGED.
9. THE TRUSS ENGINEER SHALL COORDINATE LOCATIONS OF ROOFTOP UNITS WITH THE MECHANICAL CONTRACTOR AND DESIGN THE TRUSSES AND THEIR CONNECTIONS FOR LOADS IMPOSED ON THEM PER LOADING CRITERIA SHOWN ON S001 INCLUDING WIND LOADS.
10. SEE S002 FOR COMPONENT AND CLADDING WIND PRESSURES.
  - 1) " HJ " INDICATES A WOOD HIP JACK.
  - 2) " JT " INDICATES A WOOD JACK TRUSS.
  - 3) " GT " INDICATES A WOOD GIRDER TRUSS.
11. UNLESS A MOISTURE BARRIER IS INSTALLED, ALL WOOD IN CONTACT W/ CONCRETE OR MASONRY, SHALL BE PRESSURE TREATED.
12. MECHANICAL CONTRACTOR TO COORDINATE W/ STRUCTURAL ENGINEER THRU SHOP DRAWINGS FOR APPROVAL OF EACH PENETRATION PRIOR TO INSTALL. TRUSS TO BE DESIGNED FOR ADDITIONAL 130" PER LINEAL FOOT IN-PLANE AT BOTTOM CHORD.

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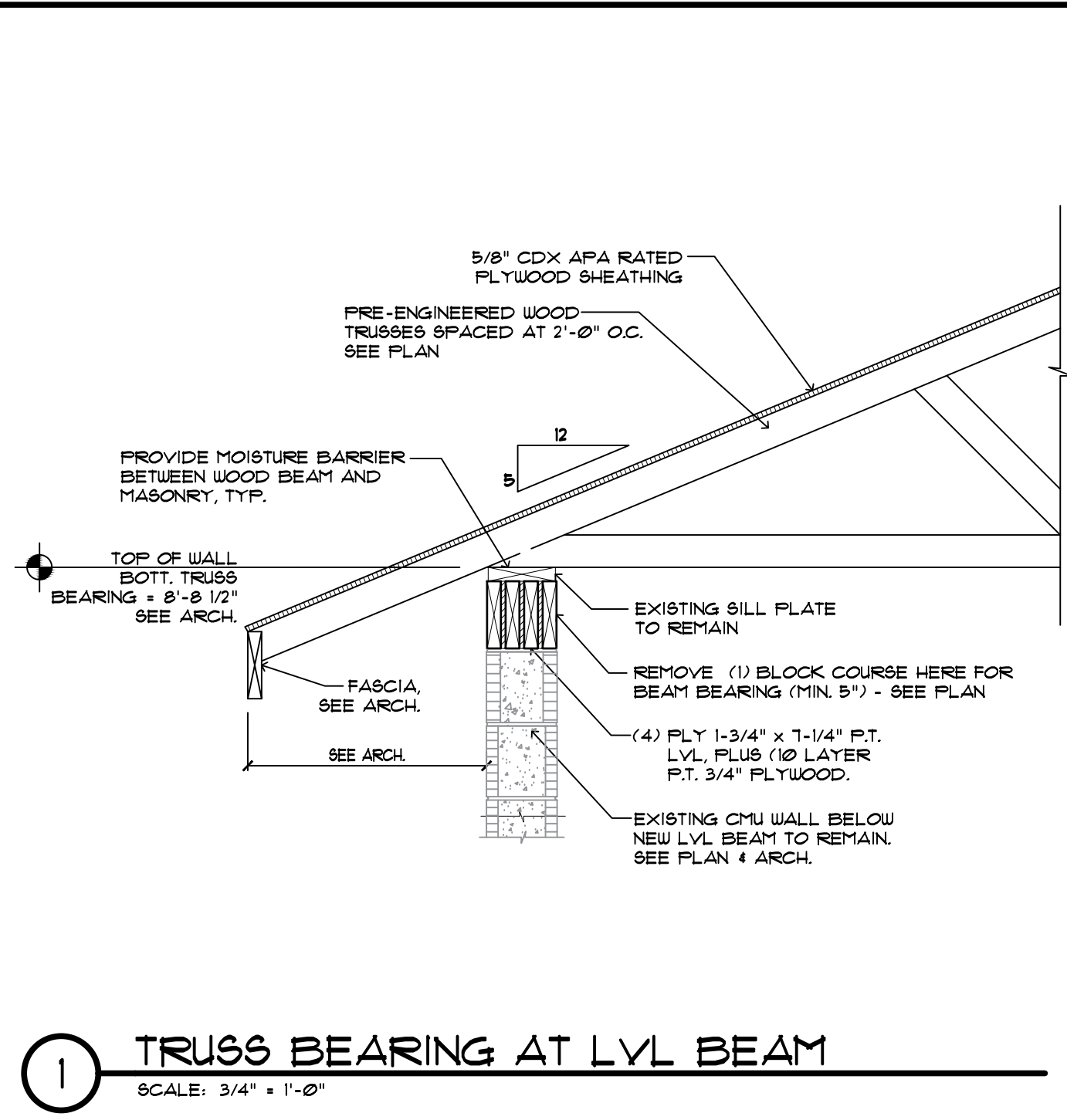
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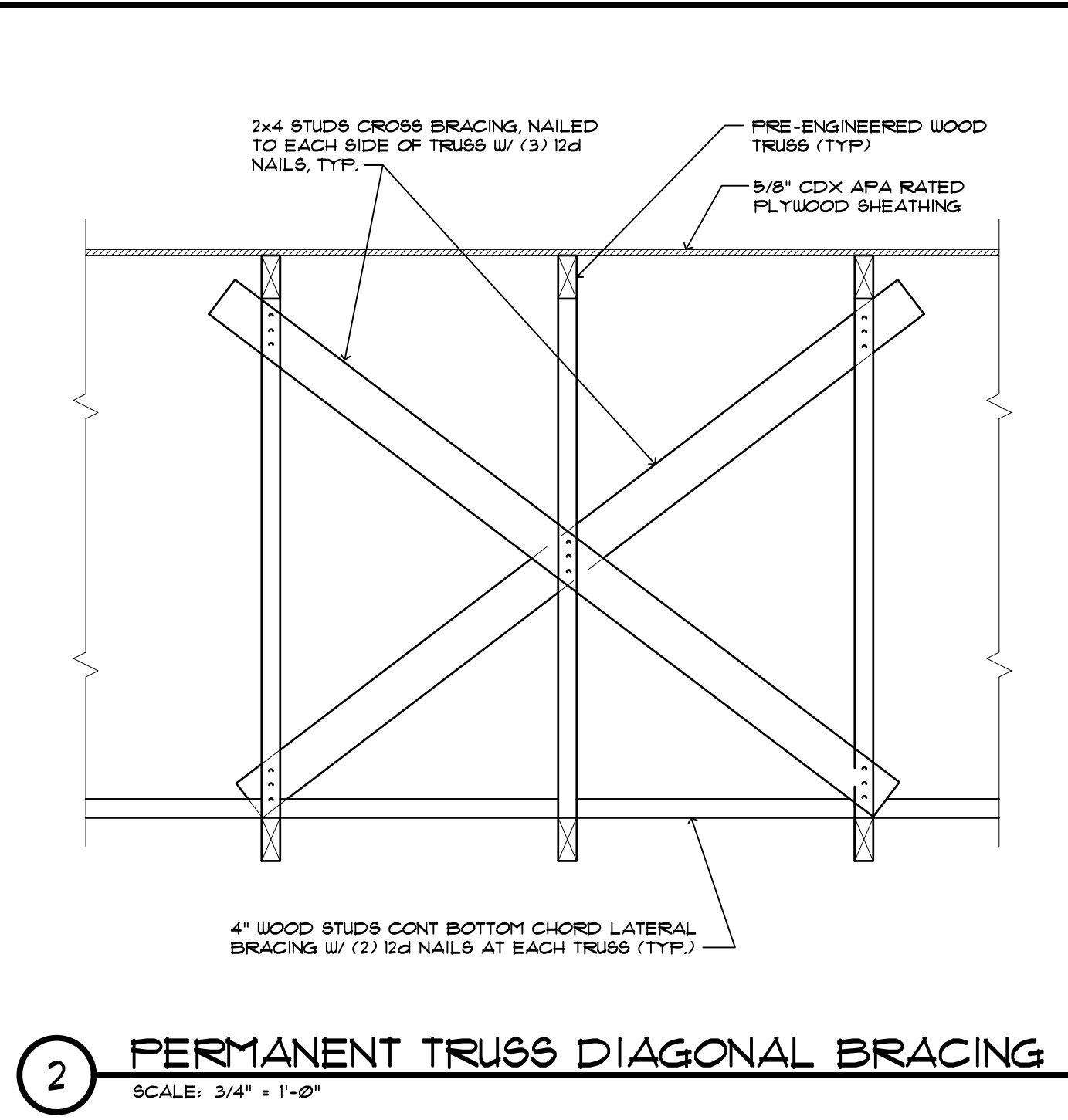
PROJECT NO.: 23002020  
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PROJECT MANAGER: T98  
CHECKED BY: KJB  
DATE: 12-28-23  
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S102  
ROOF FRAMING PLAN

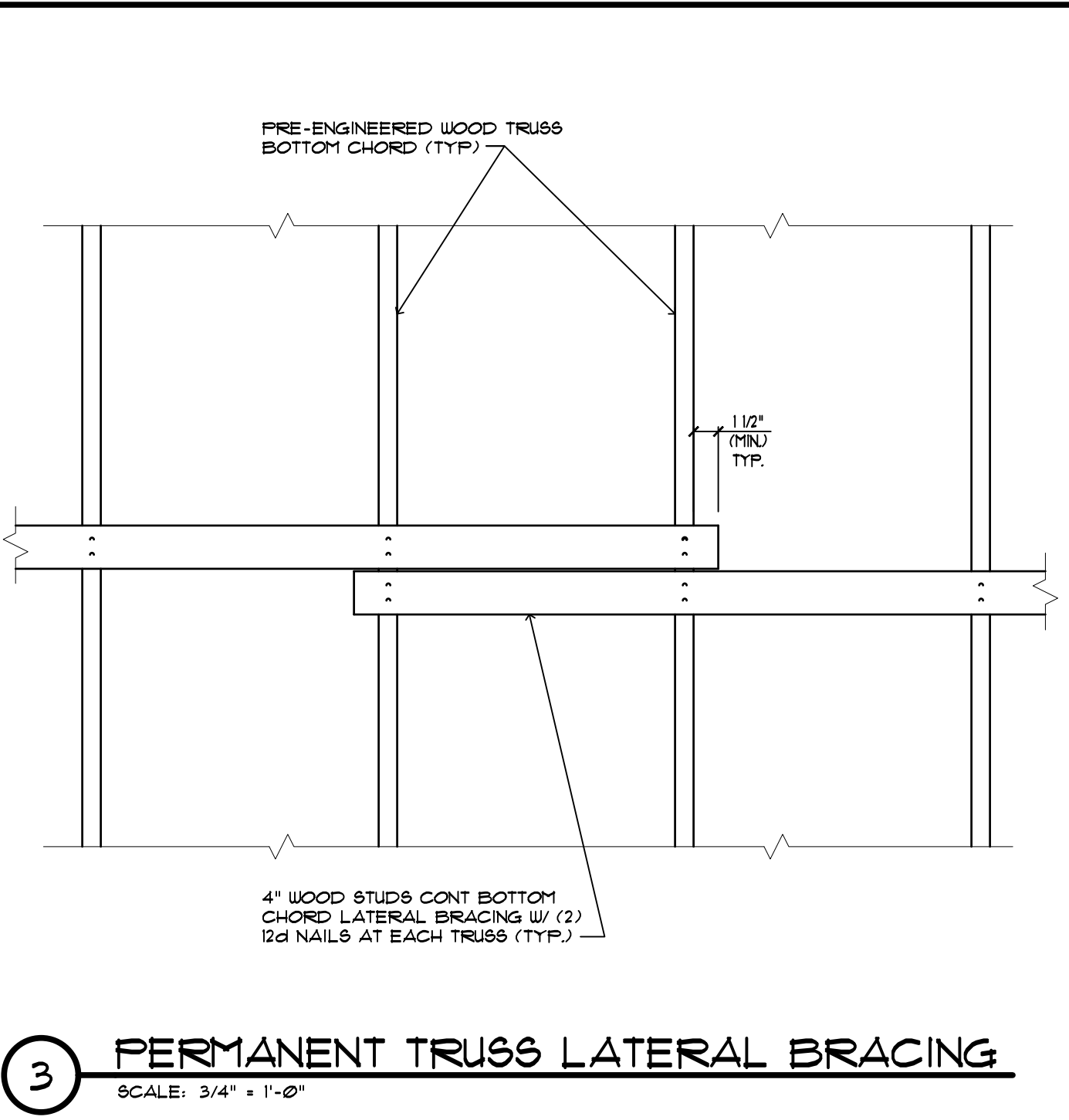




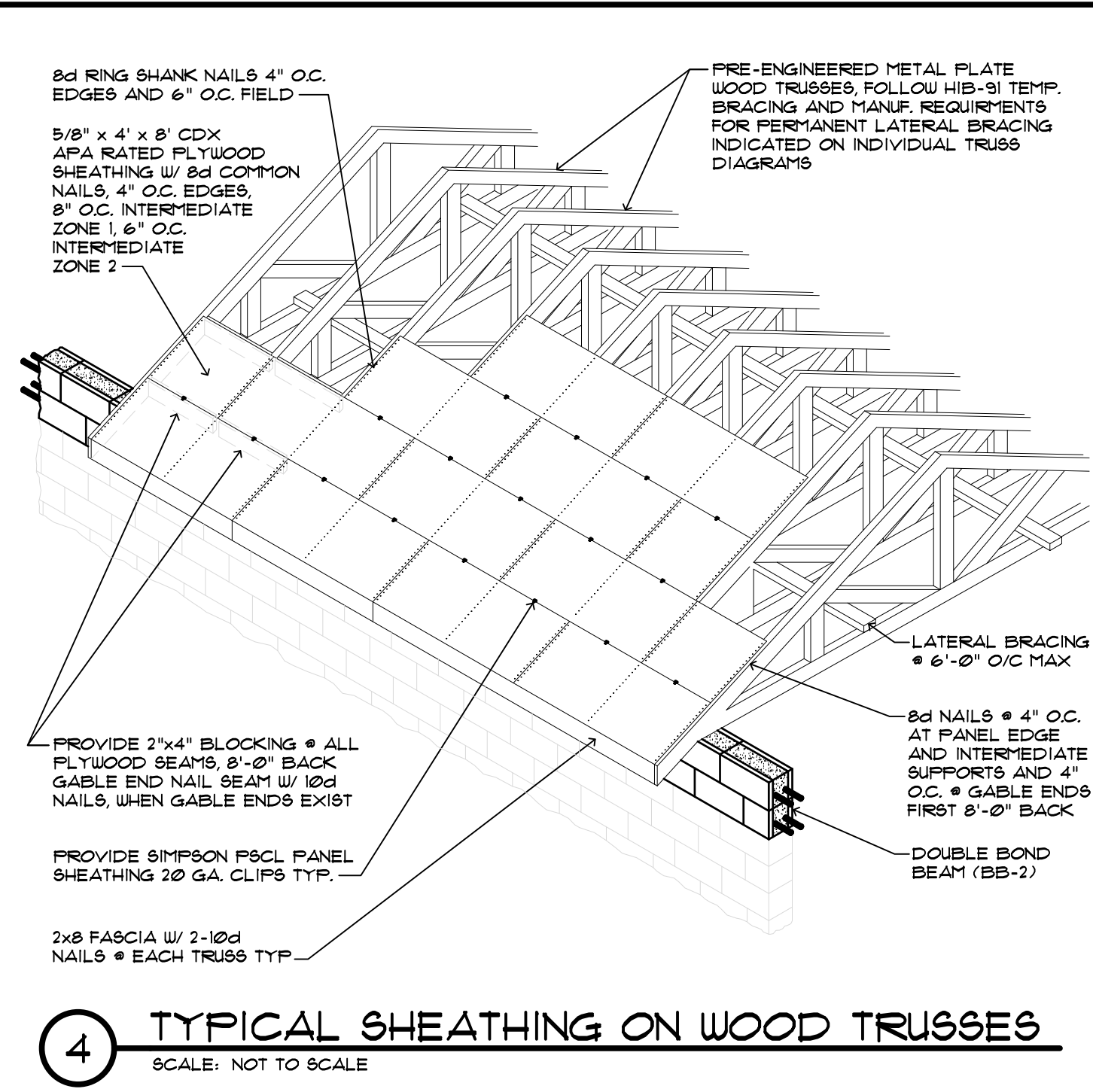
1 TRUSS BEARING AT LVL BEAM  
SCALE: 3/4" = 1'-0"



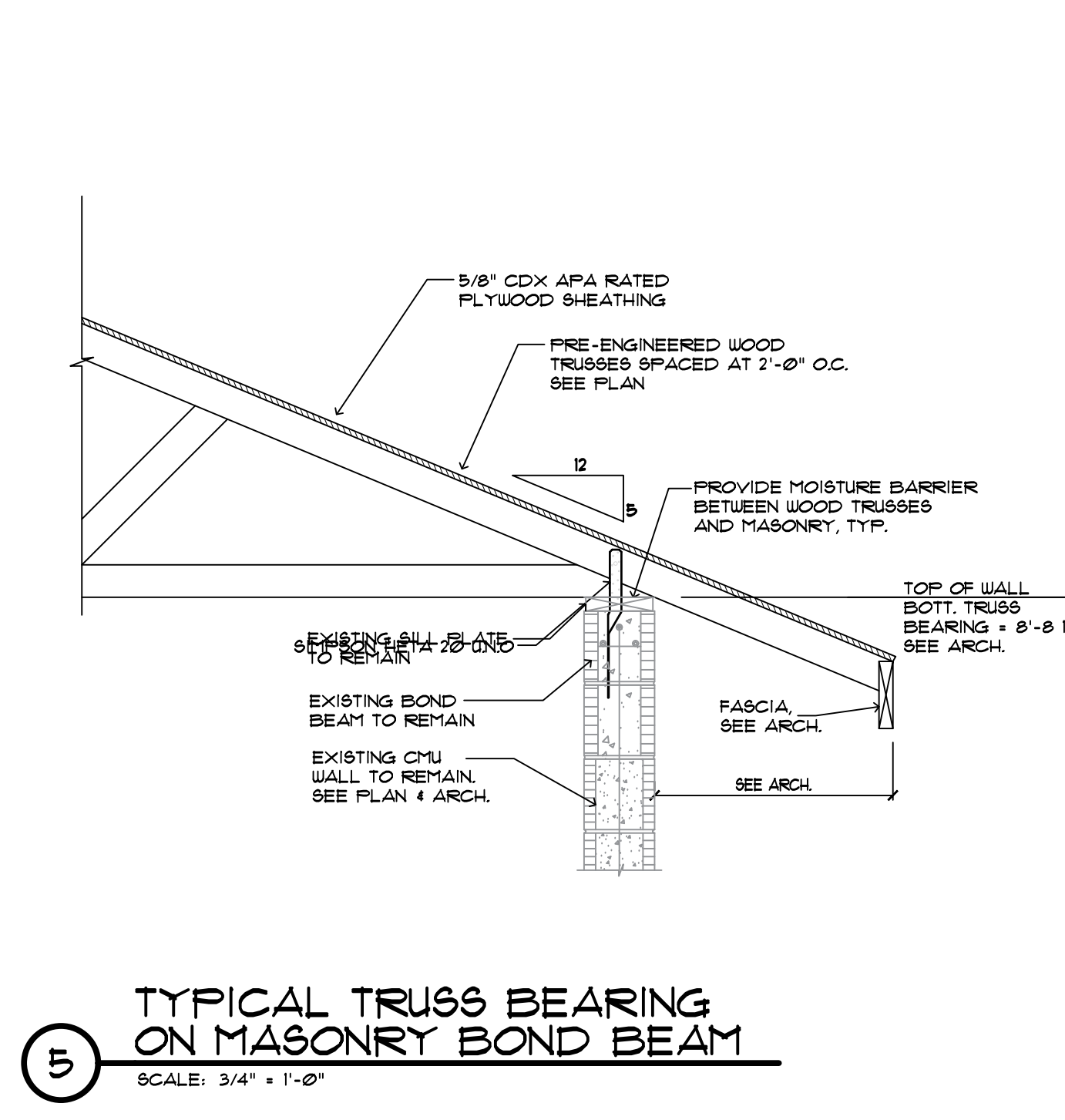
2 PERMANENT TRUSS DIAGONAL BRACING  
SCALE: 3/4" = 1'-0"



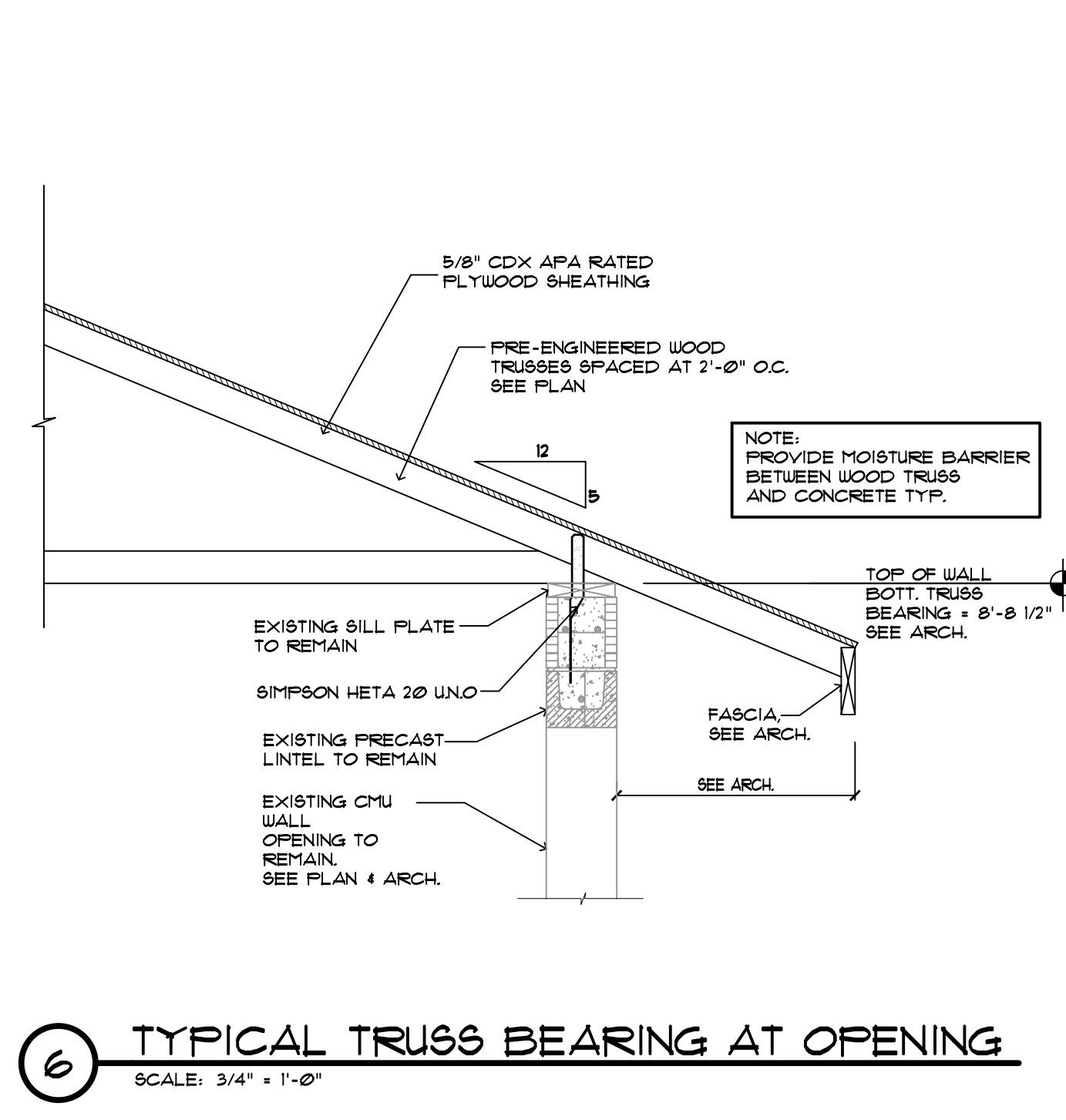
3 PERMANENT TRUSS LATERAL BRACING  
SCALE: 3/4" = 1'-0"



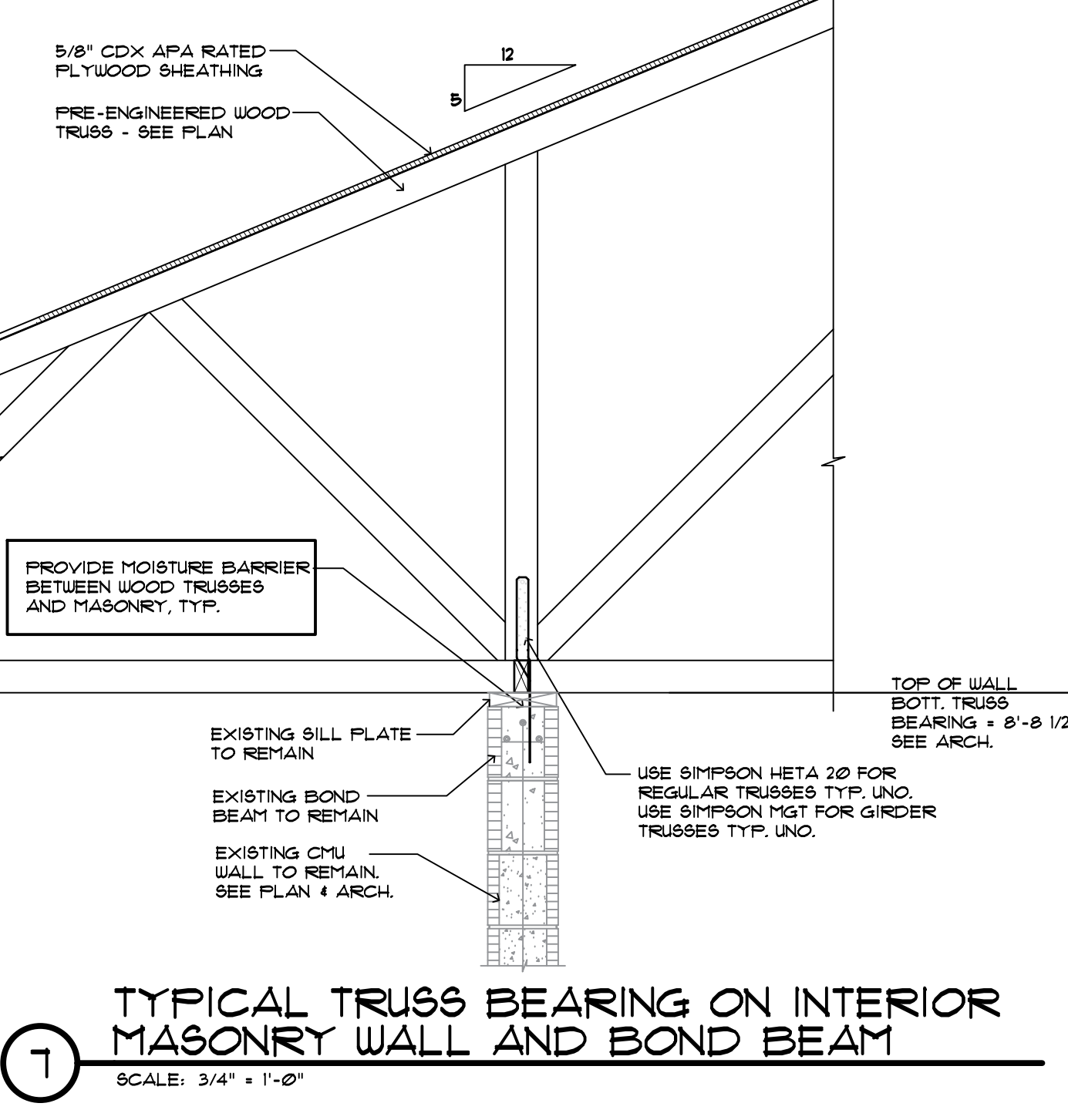
4 TYPICAL SHEATHING ON WOOD TRUSSES  
SCALE: NOT TO SCALE



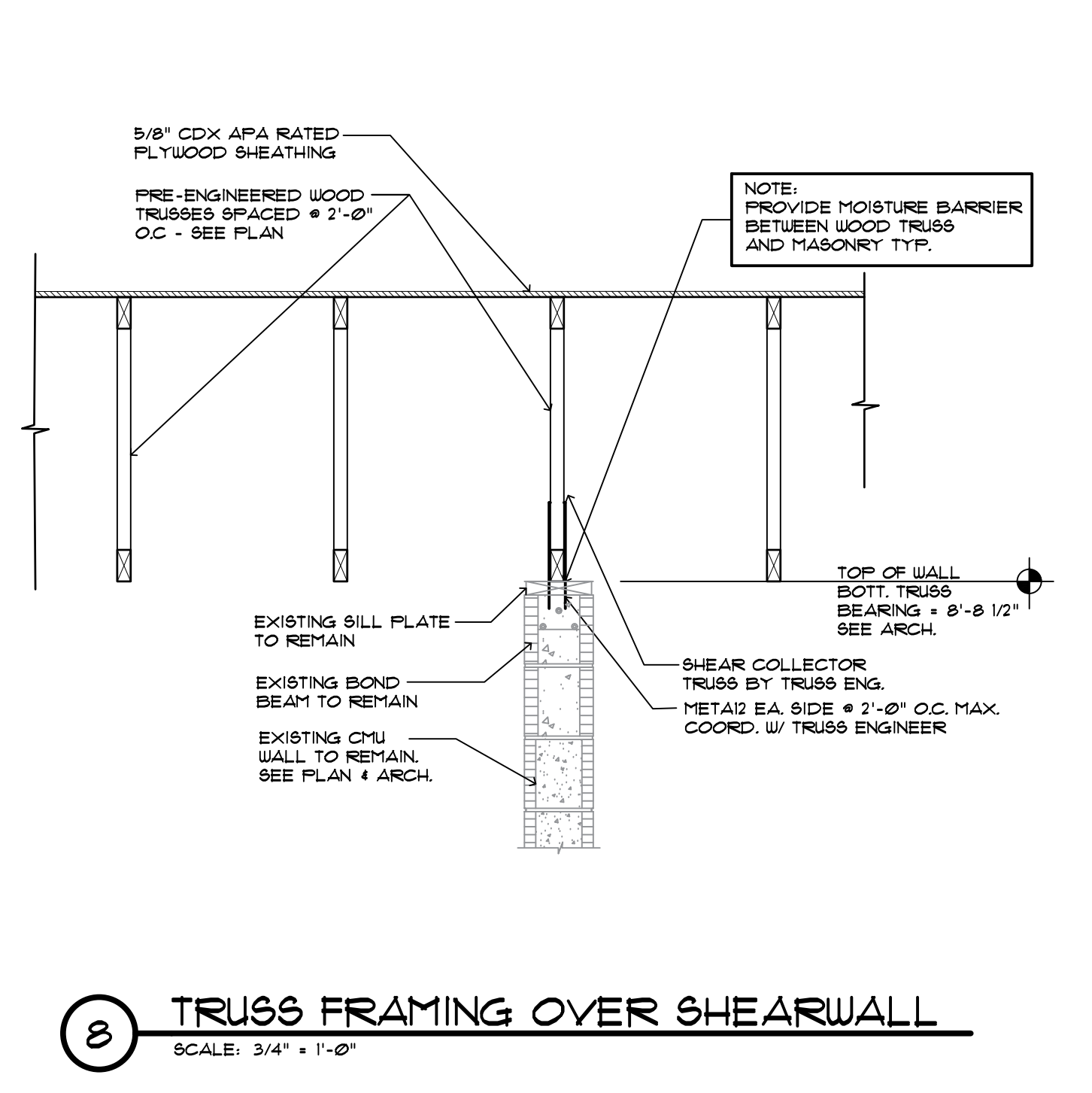
5 TYPICAL TRUSS BEARING ON MASONRY BOND BEAM  
SCALE: 3/4" = 1'-0"



6 TYPICAL TRUSS BEARING AT OPENING  
SCALE: 3/4" = 1'-0"



7 TYPICAL TRUSS BEARING ON INTERIOR MASONRY WALL AND BOND BEAM  
SCALE: 3/4" = 1'-0"



8 TRUSS FRAMING OVER SHEARWALL  
SCALE: 3/4" = 1'-0"

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PROJECT MANAGER:	T99
CHECKED BY:	KJB
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**S406**  
TYPICAL ROOF  
FRAMING DETAILS